LB/DON/25/03

EFFECTS OF ECCENTRIC CORES ON BEHAVIOUR OF TALL BUILDINGS

THESIS SUBMITTED TO THE DEPARTMENT OF CIVIL ENGINEERING IN FULFILMENT OF THE REQUIREMENT FOR THE DEGREE OF MASTRE OF ENGINEERING



T.M.D. Fernando

26 APK LJU4

Supervised By Dr. M. T. R. Jayasinghe

624 "03" 69.032.22:624.07

77711



DEPARTMENT OF CIVIL ENGINEERING UNIVERSITY OF MORATUWA SRI LANKA

January 2003



77711

Declaration

I, Manoj Fernando, hereby declare that the content of the thesis is the original work carried out over a period of 15 months at the Department of Civil Engineering, University of Moratuwa. Whenever others' work is included in this thesis, it is appropriately acknowledged as a reference.



University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk



Abstract

The latest advances of concrete technology, efficient structural forms as well as construction techniques make the new trends in the construction of tall buildings in commercial city centres of the developing countries. This is generally considered as an economical solution for the optimisation of the usable land space, especially in the countries where steel is an imported material. The planning of the tall buildings will have to fulfil the requirement of many parties, therefore, the eccentric arrangement cores in the buildings is a common situation for the structural engineers. Since the developments in concrete technology make the latest tall buildings much more taller and lighter than the earlier buildings, the behavioural studies on non-symmetric core structures are very important for various decisions made by structural engineers.

The eccentric core structures are subjected to torsional moments when resisting the lateral loads due to the eccentricity. The cores are also subjected to warping effects due to the torsional moments, therefore, the additional vertical stresses will be created. This could affect the behaviour of the building of the magnitudes of these vertical stresses are depended on eccentricity, type of loading pattern of vertical distribution of the loads etc.

This study was carried out to determine the effects of non-symmetric cores on behaviour of tall buildings. A detail case study was carried out for the buildings from twenty to thirty storey ranges for various loading conditions with different eccentricity levels to achieve the objectives of the study. The results of this study shows that it is not prudent to design buildings with eccentric cores, simply because it could be designed for the lateral loads of lower magnitude. It is shown that such structures could be subjected to severe stresses under stronger earthquakes. This could be the reason for the failure of the major structural elements and it could be the cause for the collapse of the buildings as well. Therefore, limiting the eccentricity of cores are advisable and it is prudent to have a secondary system to protect the building by improving the ductility of the building significantly, with special reinforcement details.

Key words:

Tall buildings, concrete cores, earthquake forces, wind forces, torsional moments

H

Acknowledgements

The author wishes to extend his sincere gratitude to Vice Chancellor, Dean Engineering of Moratuwa University and the Asian Development Bank for considering the research proposal favourably and granting the necessary funds.

The author is immensely grateful to the supervisor, Prof. M T R Jayasinghe of the Department of Civil Engineering for his guidance and support.

Acknowledgements are due to Prof (Mrs) N Ratnayake, (Director Postgraduate Studies), Prof. Anada Jayawardena (Head, Department of Civil Engineering), Dr S A S Kulathilake (Research Coordinator Department of Civil Engineering), and the other lecturers for the positive attitude they adopted in promoting this research project.

Assistance offered by Mr Madanayake of the Building Science Laboratory, Mr G Waidyaratne (computer systems analyst), and the other members of the staff at the computer laboratory is gratefully acknowledged. The assistance offered by fellow research students and technical assistants, in particular, Damith Dishantha. K Malnayake, P T R S Sugathadasa, is also gratefully acknowledged.

Finally, the author wishes to thank all those who contributed to the completion of this project.

TMD Fernando

February 2002

CONTENT

	Page
Declaration	i
Abstract	ii
Acknowledgements	iii
Contents	. iv
List of tables	viii
List of figures	х

Chapter 1

Chapter 2

1

INTRODUCTION

1.1	General	1
1.2	Objectives of the study	2
1.3	Methodology	2
1.4	Main findings of the study	3
1.5	An overview of chapters	4

University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk

LITERATURE REVIEW

2.1	Gener	al	5
2.2	Tall buildings		
	2.2.1	Definition	6
	2.2.2	Development of tall buildings	6
2.3	Struc	tural forms for tall buildings	7
	2.3.1	Rigid frame structures	8
	2.3.2	Shear wall structures	8
	2.3.3	Wall frame structures	9
	2.3.4	Core structures	9
2.4	The behaviour of tall buildings		9
2.5	5 Lateral forces on tall buildings		11
	2.5.1	Wind loads	12
	2.5.2	Earthquake loads	14

			Page
2.6	Analy	sis and design of tall buildings	15
	2.6.1	Design for wind load	15
		2.6.1.1 Static Method	16
		2.6.1.2 Gust factor method	16
	2.6.2	Design for earthquake loads	16
		2.6.2.1 Uniform building code (UBC, 1985) method	17
		2.6.2.2 Australian Standard (AS1170.4, 1993)	
		Method	18
	2.6.3	Importance of reinforce detailing	18
2.7	Comp	outer models for tall buildings and core for the	
	analy	vsis	19
	2.7.1	Computer modeling of shear walls	20
	2.7.2	Computer modeling of cores	. 21
	2.7.3	Computer software used	21
2.8	Sumr	University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk	22

Chapter 3

ECCENTRIC CORES IN TALL BUILDINGS

3.1	Gener	ral	32
3.2	Analysis of tall buildings with eccentrically placed cores		
3.3	Eccen	tricity of the buildings	34
	3.3.1	Eccentricity for wind load	35
	3.3.2	Eccentricity for earthquake loads	35
3.4	Vertio	cal distribution of lateral loads	35
	3.4.1	Vertical distribution of the wind force	36
	3.4.2	Vertical distribution of the earthquake force	36
3.5	Sumn	nary	38

v

C	ha	pt	er	4
---	----	----	----	---

CASE STUDY

4.1	Introd	Introduction	
4.2	Case s	study	43
	4.2.1	Details of the building	45
	4.2.2	Structural form used	46
4.3	Arrar	ngement of the cores eccentrically	46
4.4	Comp	outer models	47
4.5	Analy	sis of structures	48
	4.5.1	Wind load analysis (case – 1)	49
	4.5.2	Earthquake analysis (case – 2)	49
	4.5.3	Wind load analysis with design earthquake	
		eccentricities	52
	4.5.4	Earthquake analysis (case – 4)	52
4.6	Sumn	nary	53
Chapter 5		University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk	
RESULTS	OF THE	E COMPUTER ANALYSIS	
5.1	Gene	ral	67

5.2	Resul	ts of computer analysis	68
	5.2.1	Case 1 – wind load analysis	68
	5.2.2	Case 2 – earthquake analysis for smaller earthquake	
		loads	71
	5.2.3	Case 3 – wind loads at earthquake eccentricity	74
	5.2.4	Case 4 – earthquake analysis for stronger earthquake	
		loads	76
	5.2.5	Rotation of the buildings	79
5.3	Analy	ysis of results	79
5.4	Discu	ssion	84
5.5	Sumr	nary	86/
			in the second second
			,

Page

87

Chapter 6

CONCLUSIONS AND FUTURE WORKS

References

89

Appendices

٣

Appendix A	The typical preliminary design calculation for 20-storey	
	Building	92
Appendix B	Evaluation of the shear centres and mass centres for 20-	
	storey building	105
Appendix C	Evaluation of the gust factor and wind load tall buildings	110
Appendix D	Evaluation of the earthquake base shear force and vertical	
	distribution of the base shear force	119
Appendix E	Results of the of the computer simulations	129



University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk

LIST OF TABLES

Number

Chapter 2

- 2.1 Human perception levels with respect to the acceleration
- 2.2 Seismic probability zone factors

Chapter 4

- 4.1 The summary of the buildings used for the case study
- 4.2 Details of the buildings
- 4.3 The gust factors evaluated for the three different buildings
- 4.4 The Base shear forces due to minor earthquake
- 4.5 The earthquake design eccentricity for 20-storey building
- 4.6 The earthquake design eccentricity for 25-storey building
- 4.7 The earthquake design eccentricity for 30-storey building
- 4.8 The Base shear forces due to strong earthquake

Chapter 5

5.1 The maximum stresses at bottom of the open cores when eccentricity changed from 0% to 20%

www.lib.mrt.ac.lk

Electronic Theses & Dissertations

- 5.2 The maximum biaxial design bending moment and axial force on corner columns under open core arrangement
- 5.3 The maximum stress at bottom of the partially closed cores when eccentricity changed from 0% to 20%
- 5.4 The design bending moment and axial force on corner columns under partially closed core arrangement
- 5.5 The maximum stresses at bottom of the open cores when eccentricity changed from 0% to 20%
- 5.6 The maximum biaxial design bending moment and axial force on corner columns under open core arrangement

Number

- 5.7 The maximum stress at bottom of the partially closed cores when eccentricity changed from 0% to 20%
- 5.8 The maximum biaxial design bending moment and axial force on corner columns under partially closed core arrangement
- 5.9 The maximum stresses at bottom of the open cores when eccentricity changed from 0% to 20%
- 5.10 The biaxial design bending moment and axial force on corner columns under open core arrangement
- 5.11 The maximum stress at bottom of the partially closed cores when eccentricity changed from 0% to 20%
- 5.12 The biaxial design bending moment and axial force on corner columns under partially closed core arrangement
- 5.13 The maximum stresses at bottom of the open cores when eccentricity changed from 0% to 20% for moderate earthquake
- 5.14 The biaxial design bending moment and axial force on corner columns under open core arrangement
- 5.15 The maximum stress at bottom of the partially closed cores when eccentricity changed from 0% to 20%
- 5.16 The biaxial design bending moment and axial force on corner columns under partially closed core arrangement
- 5.17 The rotations of the buildings
- 5.18 Variation of stresses when eccentricity was changed from 0% to 20%
- 5.19 The comparison of the stress variation in case-2 and case-3
- 5.20 The maximum tensile and compressive stresses due to stronger earthquake loads
- 5.21 Comparison of maximum stresses due to smaller and stronger earthquake loads with 20% eccentricity level



LIST OF FIGURES

Number

Chapter 2

- 2.1 Moment resisting frame
- 2.2 Typical shear wall structure
- 2.3 Wall frame structure
- 2.4 Core structure
- 2.5 Deflection, moment, and forces caused by external shear on rigid frame structure
- 2.6 Behaviour of couple shear wall
- 2.7a Wall subjected to uniformly distributed horizontal loading
- 2.7b Frame subjected to uniformly distributed horizontal loading
- 2.7c Wall frame structure subjected to horizontal loading
- 2.8a Typical deflection diagram of lateral loaded wall-frame structure
- 2.8b Typical moment diagrams for components of wall-frame structure
- 2.8c Typical shear diagrams for components of wall-frame structure
- 2.9 Variation of steel requirement with the height for steel building
- 2.10 Pressure distribution on circular and rectangular building due to wind

Х

- 2.11 Load displacement relationship of SDOF system
- 2.12a Axially concentric shear wall

2.12b Equivalent column

- 2.13a Axially eccentric shear wall
- 2.13b Equivalent column
- 2.14a Couple shear walls
- 2.14b Equivalent wide column model
- 2.14c Equivalent uniform beam model
- 2.15a Non-planner shear wall assembly
- 2.15b Equivalent flexural column
- 2.16a Closed section shear wall assembly
- 2.16b Equivalent Flexural-Torsional column

Number

Chapter 3

- 3.1 Eccentric arrangement of cores inside the service core
- 3.2 Undesirable arrangement of service cores
- 3.3 Pressure distribution on object due to skew flow of streamlines
- 3.4 Partially closed core section
- 3.5 Twisting of non-symmetric structure about shear centre
- 3.6 Geometric eccentricity for earthquake analysis
- 3.7 Laterally applied static earthquake forces on building

Chapter 4

- 4.1 The selected grid arrangement for the buildings
- 4.2a The arrangement of the service core of the 20-storey building from level-1 to level-12
- 4.2b The arrangement of the service core of the 20-storey building from level-12 to level-20
- 4.3a The arrangement of the service core of the 20-storey building from level-1 to level-13
- 4.3b The arrangement of the service core of the 20-storey building from level-1 to level-12
- 4.3a The arrangement of the service core of the 20-storey building from level-1 to level-12
- 4.4a The plan view of the non-symmetric arrangement of the 20-storey building
- 4.4b The plan view of the non-symmetric arrangement of the 30-storey building
- 4.5a The 3-D model of the non-symmetric arrangement of the 20-storey building with open cores
- 4.5b The 3-D model of the non-symmetric arrangement of the 30-storey building with open cores



Number

- 4.6a The 3-D model of the symmetric arrangement of the 20-storey building with open cores
- 4.6b The 3-D model of the symmetric arrangement of the 30-storey building with open cores
- 4.7 The plan view of the symmetric arrangement of the 30-storey building
- 4.8a The arrangement of the open cores in 20-storey building
- 4.8b The arrangement of the partially closed cores in 20-storey building
- 4.9 The 3-D model of the partially closed core arrangement of the 20-storey building
- 4.10a The local arrangement of the open core
- 4.10b The local arrangement of the partially closed core

