

**TO PERFORM AN ECONOMIC ANALYSIS OF
MICROTUNNELING WITH SPECIAL REFERENCE
TO THE COLOMBO CITY**

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Declaration

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Abstract

The city growth taken place with the economies, there had been a development of the infrastructure facilities such as water, electricity, sewer, storm water, gas, and telecommunication. Some of these services were in the form of overhead lines and some were underground. All of these services had to be provided in congested areas where no proper planning had been done in the past. Therefore, it is a tedious task for service providers to carry out open cut excavation and lay the necessary service lines as it would interfere with other facilities provided by the state such as roads and railways. In order to avoid public disturbances, various techniques have been developed by Engineers. One of the feasible solutions is Microtunneling technology without performing open cut excavations. Although tunnelling is expensive, comparatively, it causes minimum disturbances to the general public. It also minimises the interruption of other facilities and interference with routine traffic flows are minimum. Further in the recent past microtunnelling was used in a few project in the Colombo city. Therefore, it is important to perform research on the technology used and cost of construction to give information and guidance for future projects.

The study is a combination of case study method and document analysis of available survey data. Three case studies based on construction projects in Sri Lanka were chosen for the study. Qualitative interviews were also conducted with five key decision makers of these projects in order to collect unquantifiable and unmeasurable data. The findings revealed that even though the direct costs incurred in open-cut constructions are lower than Microtunneling constructions, when all the indirect costs are considered the total cost of open-cut construction method is significantly higher, unmeasurable and the cost-impact is very long-term compared with the open-cut construction method. The pipe jacking and thrust boring methods have been used in Colombo city projects. The microtunnelling is successful without much technical issues. This research study will provide insightful cost evaluations in terms of the direct and indirect costs (total cost) of the Microtunneling projects conducted in Sri Lanka, which in turn will support the decision-making process.

Table of Contents

Declaration.....	ii
Acknowledgements.....	iii
Abstract.....	iv
1 INTRODUCTION.....	1
1.1 Background to the study.....	1
1.2 Research problem.....	2
1.3 Research objectives.....	2
1.4 Significance of the study.....	3
1.5 Research methodology.....	3
1.6 Thesis overview.....	4
2 LITERATURE REVIEW.....	5
2.1 Types of tunnelling.....	5
2.2 Traditional open trench technology.....	6
2.3 Trenchless technology.....	9
2.4 History of microtunneling.....	20
2.5 Microtunneling technology.....	21
2.6 Economics of Microtunneling.....	23
2.6.1 Direct costs.....	25
2.6.2 Indirect costs.....	26
2.6.3 Socio-economic inconvenience costs (External costs).....	27
2.7 Summary.....	30
3 RESEARCH METHODOLOGY.....	31
3.1 Research design.....	31
3.2 Nature of the inquiry.....	31
3.3 Data collection.....	33
3.4 Data analysis.....	34
3.5 Ethical considerations.....	34
3.6 Summary.....	34
4 CASE STUDY OF MICROTUNNELING IN COLOMBO CITY.....	35
4.1 History and projects.....	35
4.2 Microtunneling projects in Sri Lanka.....	36
4.2.1 Case Study 01.....	36
4.2.2 Case Study 02.....	38
4.3 Technology used.....	39
4.4 Methodology.....	46
4.4.1 Pipe jacking Methodology.....	46
4.4.2 HDD Methodology.....	50
4.4.3 Horizontal Pipe jacking Methodology.....	52

4.5	Economic analysis.....	54
4.6	Proposed construction sequence.....	57
4.7	Cost effectiveness.....	59
4.8	Cost comparison.....	60
4.9	Parameters for analysis for applications in Sri Lanka.....	65
5	CONCLUSIONS AND RECOMMENDATIONS.....	70
5.1	Technology.....	70
5.2	Cost parameters.....	70
5.3	Framework for critical analysis of Microtunneling in Sri Lanka.....	71
	REFERENCES	72
	Appendix 1.....	75

LIST OF FIGURES

Figure 1 - Typical open cut elements (Ormsby,2009)	6
Figure 2 - Traditional open-trenching approach (International Society for Trenchless Technologies, 2019).....	8
Figure 3 - Trenchless approach (International Society for Trenchless Technologies, 2019)	9
Figure 4 - Underground construction techniques of trenchless nature	10
Figure 5 - Typical components of utility tunnelling method	11
Figure 6 - Pipe jacking components.....	13
Figure 7 - Typical components of a pipe jacking operation	13
Figure 8 - Auger boring process	15
Figure 9 - Horizontal directional drilling method (HDD)	16
Figure 10 - Pipe ramming method	17
Figure 11 - Cut-away illustration of a microtunneling project (Raymond L. Sterling, 2018)	19
Figure 12 - Microtunnel boring machine (MTBM)	19
Figure 13 - Micro tunnel boring machine (MTBM)	20
Figure 14 - Microtunneling system layout (Nicholas, 2014).....	21
Figure 15 - Typical microtunneling site.....	22
Figure 16 -Typical arrangement of microtunnelling jacking shaft	22
Figure 17 - Comparison of cost-breakdown for open-cut and trenchless methods.	24
Figure 18 - Cost-effective route selection.....	24
Figure 19 - Cost identification for underground utility project (Woodroffe & Ariaratnam, 2008).....	26
Figure 20 - Cost components of indirect and social costs	28
Figure 21 - Social impacts and cost indicators according to Gilchrist and Allouche (2005).....	29
Figure 22 - Social cost categories according to Rahman, Newton and Vanier (2005)	30
Figure 23 Schematic Diagram of the Colombo Sewer System.....	36
Figure 24 Sewer System of Colombo	37
Figure 25 - Sub catchment area map Torrington tunnel	38
Figure 26 - Proposed site layout of the Torrington tunnel.....	38
Figure 27 - Typical Microtunneling site	39
Figure 28 - Route Plan of Wanathamulla Forcemain Area.....	40
Figure 29 Installation of Jacking Machine.....	41
Figure 30 Jacking machine is in operation	41
Figure 31 -Carryover of TBM from one place to another	42
Figure 32 - Preparation of driving shaft at Manhole No.18.....	43
Figure 33 - Checking the pipe alignment.....	43

Figure 34 - Model conceptualisation	45
Figure 35 - Model conceptualisation	45
Figure 36 - setting-up of guide rails and jacking rig.....	46
Figure 37 - setting up of TBM Shield	46
Figure 38 - Jacking of TBM Shield	47
Figure 39 - Jacking of product pipe	48
Figure 40 - VC pipe Jacking in operation	49
Figure 41 - Jacking of steering head and pilot tubes from jacking pit.....	49
Figure 42 - Pilot Hole Drilling	50
Figure 43 – Reaming.....	50
Figure 44 – Pipe Pull Back	51
Figure 45 Jacking alignment of Torrington Tunnel.....	52
Figure 46 - General layout of Torrington Tunnel	53
Figure 47 - R A D Mel mawatha Bullers road junction.....	66
Figure 48 - Vajira road / R A D Mel mawatha junction	66
Figure 49 - Thimbirigasyaya road / Sri Sambuddhathva Jayanthi mawatha junction .	66
Figure 50- Sri Sambuddhathva Jayanthi mawatha / Isipathana mawatha junction	67
Figure 51 - Bambalapitiya junction	67
Figure 52 - Ayurveda junction	67
Figure 53 - Devi Balika junction	68
Figure 54 - Kanaththa junction	68
Figure 55 - Panchikawaththa junction	68

LIST OF TABLES

Table 1 - Type of tunneling (compiled by author based on several sources of literature).....	5
Table 2 - Characteristic of utility tunnelling.....	11
Table 3 - Characteristic of pipe jacking.....	12
Table 4 - Characteristic of horizontal auger boring.....	14
Table 5 - Characteristics of horizontal directional drilling method.....	15
Table 6 - Pipe ramming method.....	17
Table 7 - Microtunneling method.....	18
Table 8- Comparison of cost factors between open-cut and trenchless technology	25
Table 9 - Methodologies used in research involving buried municipal infrastructure construction projects.....	32
Table 10 - Sri saddarma mawatha 375mm dia. pipe - open cut.....	60
Table 11 - Sri saddarma mawatha 375mmdia. pipe microtunneling.....	61
Table 12 - Maligawatte railway quarters road 600mm dia. pipe open cut.....	62
Table 13 - Maligawatte railway quarters road 600mm dia. pipe microtunneling.....	63
Table 14 - hourly distribution of traffic movements by all approach links at junction No. 5.....	75
Table 15 - hourly distribution of traffic movements by all approach links at junction No. 7.....	76
Table 16 - hourly distribution of traffic movements by all approach links at junction No. 6.....	76
Table 17 - hourly distribution of traffic movements by all approach links at junction No. 56.....	76
Table 18 - hourly distribution of traffic movements by all approach links at junction No. 44.....	76
Table 19 - hourly distribution of traffic movements by all approach links at junction No. 27.....	76
Table 20 - hourly distribution of traffic movements by all approach links at junction No. 46.....	76
Table 21 - hourly distribution of traffic movements by all approach links at junction No. 38.....	76
Table 22 - hourly distribution of traffic movements by all approach links at junction No. 58.....	76

1 INTRODUCTION

1.1 Background to the study

The industrial revolution caused a mass exodus of people from rural areas to industrial cities. As a result, population in the cities increased dramatically. Large townships were established and service industries were expanded in order to sustain the increased population. These services include the industries that provide all the infrastructure facilities required for cities in order to support this huge population growth. Some services had to be laid subterranean below the ground level in order to save space in the city as the city centres were confined areas. Continued population growth in cities has necessitated the development of strong water and sewerage network, many of this can be constructed underground. It was necessary to construct these underground pipelines in parallel with the existing road network and habitation.

The traditional method of installing underground pipelines has been dig/trench along the alignment of the pipeline and manually install the pipes and joints. This method is also referred to as open-cut method or open trench method. The features of open trench require many supplementary processes such as detour of roads, storage of excavated materials on the site, backfilling and compaction, ground water management, and restoration of surfaces post the installation (Gottipati 2011). These activities that are additional to the main process of installing pipes consume time, money and disrupt the movement of vehicles and pedestrians. Furthermore, there were many other technological, economic and social disadvantages associated with open trench excavations.

Therefore, other means of providing underground services that avoid mass scale open cut excavation were needed to be developed. As a result of this challenge, various techniques were developed by engineers which minimize the disturbances to the road surfaces and the surrounding areas that would also minimize the other negative aspects of open trench excavation. These technologies are together referred to as trenchless technologies. Trenchless technologies are characterized as those that minimize the need for personnel to be working in the trench below ground level (Gottipati 2011). Among the various trenchless technologies, Microtunneling is one of the feasible alternatives to open trench excavations.

1.2 Research problem

While Microtunneling offer various advantages when compared with the open cut excavation, there are also various challenges and risks associated with Microtunneling (Salem, Elwakil, and Hegab 2017). An important issue in increasing the acceptance of Microtunneling (and other trenchless technologies) compared to the traditional open cut construction was the acceptance that various forms of direct and indirect costs involved should be considered in addition to the direct construction costs (Sterling 2018). While direct costs of trenchless technology construction projects are well understood by practitioners, indirect costs including social, economic and environmental costs are often not well understood (Ormsby 2009). As the indirect costs are not well understood, there could be a tendency for incorrect cost evaluations. Direct and indirect costs incurred in Microtunneling construction should not disproportionately increase the cost of construction and therefore cost evaluation must be carried out before implementing such projects.

Microtunneling is a relatively new technology to Sri Lanka, however, some of the key construction projects in the city of Colombo have utilized Microtunneling technology. However, there is scarcity of research conducted in the Sri Lankan context in terms of the direct and indirect costs involved in Microtunneling construction. Indirect and socio-economic inconvenience costs of these projects must form part of the total cost of a project as it assists with the successful completion of the project without expensive unforeseen costs. Implications of social, economic and the environmental costs in the Sri Lankan context would be different to those of the Microtunneling constructions in other countries. Therefore, the present study is focused on the cost evaluation of Microtunneling method of pipe laying compared to the open cut excavation.

1.3 Research objectives

The main objective of this study is to review Microtunneling applications to establish the most suitable technologies by identifying costs of main tasks. This objective will be achieved through the following sub-objectives:

1. To review the technologies used in Microtunneling in Sri Lanka and other countries.
2. To establish the cost parameters of Microtunneling.
3. To propose a framework for critical analysis of Microtunneling in Sri Lanka.

1.4 Significance of the study

This research study will serve as a support tool to municipalities of Sri Lanka and other developing countries with similar context when selecting a Microtunneling construction method. This research will provide key technical merits and drawbacks of the traditional open trenching method versus Microtunneling method. In addition, this research study will provide insightful cost evaluations in terms of the direct and indirect costs (total cost) of the Microtunneling projects conducted in Sri Lanka, which in turn will support the decision-making process.

The decision-making process lies in the hands of the municipal technical managers. Therefore, their knowledge and experience of up to date information on cost evaluations of Microtunneling technology is vitally important. This research provides insights on various forms of costs involved in the Microtunneling method and its application and use in Sri Lanka. Hence, the present study would bridge the knowledge gap in the construction industry, which would facilitate more accurate cost estimations and effective decision making.

1.5 Research methodology

The main empirical work for this study utilizes a case study approach. Case study method permits selecting information-rich cases that allow an in-depth study of the selected projects and a case study approach has the capacity to accommodate a variety of research techniques, the flexibility to select appropriate information-rich cases to study holistically, the ability to investigate a contemporary phenomenon within its real-life context.

A very few construction projects in Sri Lanka that utilized Microtunneling method were chosen for the study. Drawing on qualitative research tools, a thematic analysis of participants' data gathered from in-depth interviews and documents analysis was employed. In addition to the first hand data collected by interviewing key decision makers such as engineers of the Microtunneling construction projects, various important documents of these projects were analysed to evaluate direct and indirect costs and influencing factors associated with these projects.

1.6 Thesis overview

Chapter one of this thesis provides the introduction to the study which consist of research background, research problem, objectives, significance and a brief introduction to the methodology of the study. Chapter two provides a literature review of the chosen research domain. It discusses traditional open trench method and introduces various types of tunnelling technologies used in pipe laying and subterranean constructions. The chapter further discusses trenchless technologies and Microtunneling technology with its history and advantages. The literature review chapter then focuses on economies of Microtunneling to review literature on various forms of costs associated with Microtunneling technology.

Chapter three of the thesis discusses the case study methodology with relevant information on how the cases were chosen and the ways in which data were gathered. The chapter then discusses the analysis methods used in the present study. Chapter five of the thesis presents findings and the analysis of the data. Chapter six discusses conclusions, recommendations and further research work.

2 LITERATURE REVIEW

This chapter evaluates the extant key literature in the research domains of construction related to open trench and trenchless technology, and Microtunneling technology. The chapter begins by briefly discussing traditional open trench technology. The chapter then evaluates alternatives to open trench technology to focus on a detailed evaluation of Microtunneling technology. The chapter discusses advantages of Microtunneling technology along with a brief history of Microtunneling technology development. The chapter then focuses on economics of Microtunneling to discuss and compare various kinds of costs associated with Microtunneling technology.

2.1 Types of tunnelling

Underground installation of pipelines consists of two main approaches; (1) traditional open trench approaches, and (2) trenchless approaches. Table 1 summarises key construction methods belonged to these two methods that are used in underground installation of pipelines.

Table 1 - Type of tunneling (compiled by author based on several sources of literature)

Method	Where the method is used
Cut and cover	Shallow tunnels with wide cross sections
Drill and blast	Hard rock, mountainous areas
Soft ground TBMs: EPB machines	Soft ground, larger diameter tunnels such as metros and rail lines, ground with low permeability
Soft ground TBMs: slurry machines	Soft ground with unstable granular soils, high permeability
Hard rock TBMs: gripper, single and double shields	Varying types of rock
Immersed tube	Shallow water with soft bed conditions
Mining/NATM/sprayed concrete lining method	Competent soils and rock
Microtunneling	Diameters less than 3 metres

2.2 Traditional open trench technology

The traditional method of installing underground pipelines has been to dig/trench along the alignment of the pipeline and manually install the pipes and joints. Open trench construction (also known as conventional trenching, open cut, cut and cover, and dig and replace) is a method in which access is gained by excavation from ground level to the required level underground for the installation, maintenance or inspection of a pipe, conduit or cable (Ormsby, 2009). Typical steps involved in this operation are outlined by (Parker, 2007) and include the following:

- Cut through surface and provide side slopes (usually 2:1) and bench, wall shoring or worker shielding.
- Excavate and remove the existing pipe with attention to special handling procedure and disposal regulations.
- Provide proper pipe bedding and compaction.
- Install warning tape and or identification wire where applicable.
- Backfill the trench providing adequate cover.
- Restore the surface conditions.
- Perform the required testing prior to commissioning of the pipe.

Figure 1 depicts a cross-section of typical construction elements associated with open trench excavation.

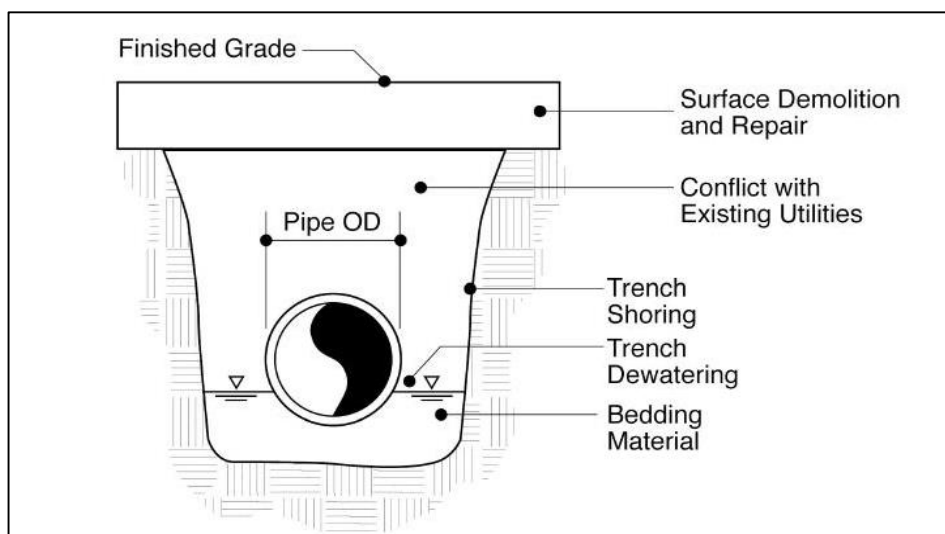


Figure 1 - Typical open cut elements (Ormsby,2009)

The features of open trench require many supplementary processes such as detour of roads, storage of excavated materials on the site, backfilling and compaction, ground water management, and restoration of surfaces post the installation. These activities that are additional to the main process of installing pipes consume time, money and disrupt the movement of vehicles and pedestrians (Gottipati, 2011). Cost of these constructions normally exceeds the direct costs to include indirect costs and socio-economic inconvenience. Accordingly, various sources of literature proclaim that traditional open trenching method has shown to be expensive and difficult to implement, particularly in congested high traffic use urban areas. The following are the most emphasised negative impacts of open trench technology (Hafez, Aziz, & Attia, 2015; Ormsby, 2009; Staheli & Hermanson, 1996).

- Vehicular/pedestrian traffic as roadways and sidewalks needed to be restricted from daily use in order to place pipes beneath them.
- Worker safety: Trench safety is a major concern for contractors when performing open-trench construction.
- Interruption of local businesses: Local businesses are likely to lose customers due to resulting traffic disruptions associated with open-trench pipe construction.
- Major inconvenience, congestion, and delays are often imposed on neighbourhoods and their residents due to open-trench pipe construction nearby.
- The increased number of pavement joints at patched surfaces necessitate frequent maintenance resulting in additional traffic impacts and higher life-cycle costs.
- Damage to existing utilities: Existing utilities near the trench are often damaged during the trench excavation and from subsequent soil settlement.
- Soil disposal: Contaminated soil is sometimes encountered during pipe construction.
- Air pollution: Fine soil particles from soil stockpiles created during open trench excavations may become airborne and cause air pollution.
- Water pollution: Water (rain or subsurface pumping discharge) can cause soil erosion and solids may runoff into streams, rivers, and storm sewers.
- Roadways: Open-trench construction often requires sawing, demolition, or removal of pavements followed by subsequent restoration. This significantly reduces pavement life by up to 40 per cent.
- Noise: Open-trench excavation requires the use of heavy equipment that produces levels of noise that cause disturbances to hospital, schools, business, and residences.

- Land defacement: Open-trench pipe construction frequently causes damage and can have adverse short-term effects on grass, trees, and other landscaping features.

Figure 2 describes the traditional open trenching scenario (International Society for Trenchless Technologies, 2019).

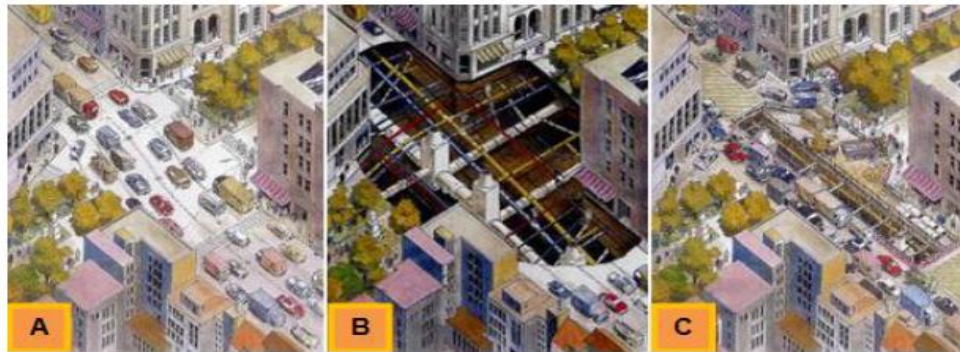


Figure 2 - Traditional open-trenching approach (International Society for Trenchless Technologies, 2019)

Plate A: Illustrates a typical city junction with traffic which is congested with vehicular and pedestrian traffic, yet flowing normally.

Plate B: Illustrates the unseen maze of existing underground service utilities beneath the roadways and walkways.

Plate C: Illustrates the chaos and disruption which occurs when the underground services need to be replaced using the traditional open trenching method. Streets have to be dug up, lanes closed, traffic backlog and unnecessary dust and noise generated.

Proper utilisation of the access points to underground constructions minimises the need to dig up streets and sidewalks, and minimise the negative impacts occurred in open-trench excavations. Taking this approach as an alternative to open trench, several methods have emerged over the years that minimise trench work. These technologies are together referred to as trenchless technologies. The following section discusses trenchless technologies in general with a special focus on Microtunneling technology.

2.3 Trenchless technology

Trenchless technology is a family of non-intrusive construction methods, materials and equipment, used for the installation, replacement or rehabilitation of underground infrastructure, while minimizing excavation and disruption at the ground surface (NASTT, 2007). Trenchless technologies are characterized as those that minimise the need for personnel to be working in the trench below ground level (Hay, 2013). Trenchless methods adopt the following procedures (Yemelin, 2011).

- Broaching a cable
- Pipeline drainage
- Transportation of clearing tools (brushed, scraper, pistons, etc.)
- Pipeline condition assessment (defects)
- Pulling in a new pipeline (liner)
- Connecting existing off-takes to houses etc.
- Disinfecting
- Commissioning the new pipeline.

Figure 3 reflects smooth flowing traffic with minimal disruption when trenchless methods are utilised.



Figure 3 - Trenchless approach (International Society for Trenchless Technologies, 2019)

The numerous trenchless methods available on the market today depend on client and contractor preference, expertise and equipment availability (R.L. Sterling, 2010). These methods differ under various factors such as the type of material and shape that the existing

ageing deteriorating host pipe consists of in case of rehabilitation projects (Halloran & Slattery, 1999; Wrobel, Szymiczek, & Wierzbicki, 2004). Chin and Lee (2005) state that there are about eighty (80) forms of trenchless methods that have been developed. A detailed diagram indicating the many forms of trenchless methods available is shown in Figure 4

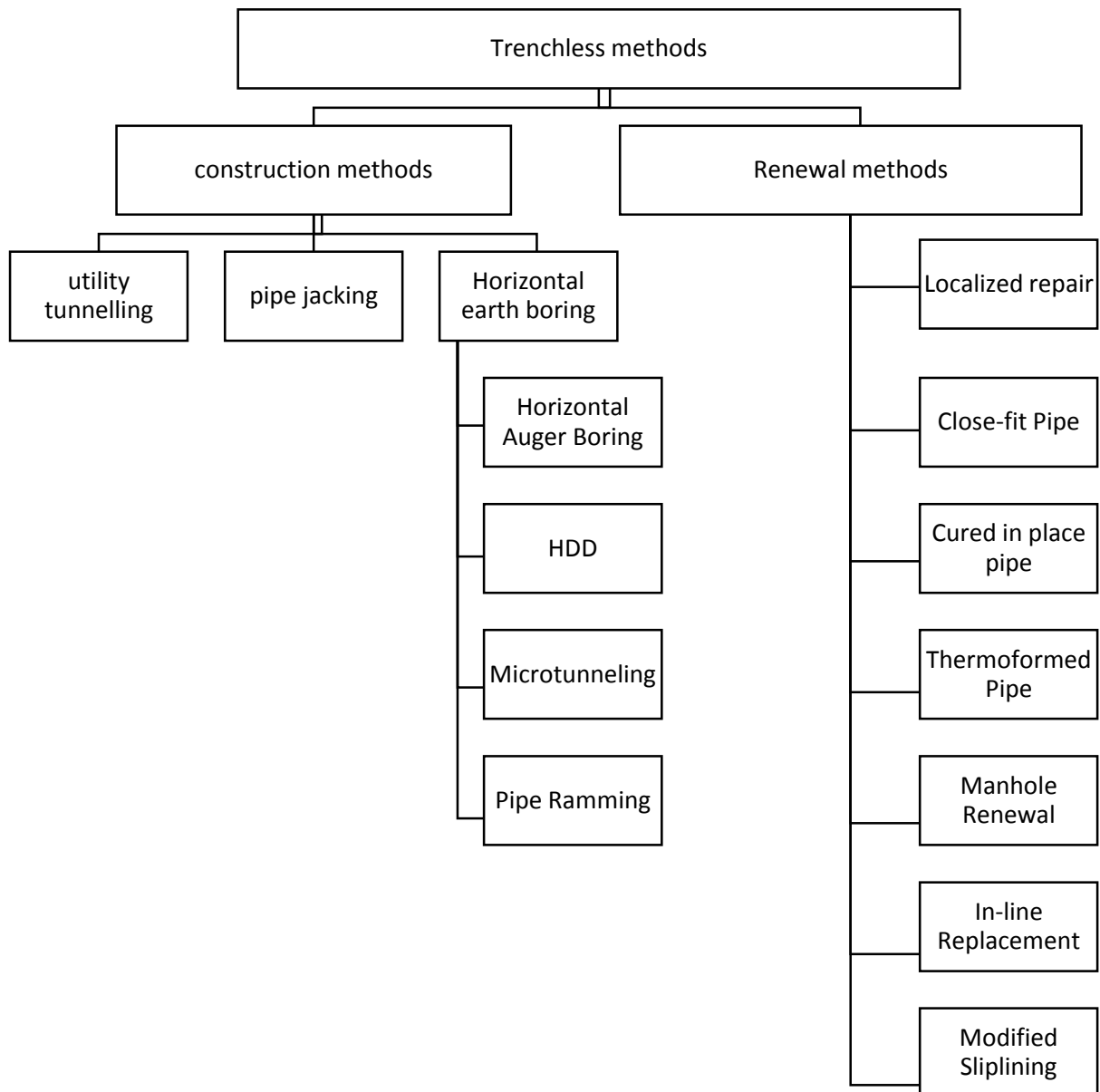


Figure 4 - Underground construction techniques of trenchless nature

The following sections briefly discuss each of these methods before focusing on Microtunneling technology.

2.3.1. Utility tunnelling method

Utility tunnelling is a trenchless construction method which requires workers to enter the borehole area. It is similar to pipe jacking. However, this technique involves liner plates to provide a shield for workers and to secure the tunnel (Sullivan, 2018).

- Performed in two steps
 - Excavation and Installation of Primary Support
 - Installation of pipe (Secondary Support/Liner System)
- Product pipe sizes 42” and larger
- Limitations on length and size based on logistical considerations and safety

Table 2 demonstrates characteristics of utility tunnelling and Figure 5 demonstrates the typical components of the Utility tunnelling method

Table 2 - Characteristic of utility tunnelling

Method	Diameter Range (inches)	Typical Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Utility Tunnelling	42” & large	1,600	RCP, GRP, Steel	Pressure and Gravity Pipelines	~ 1

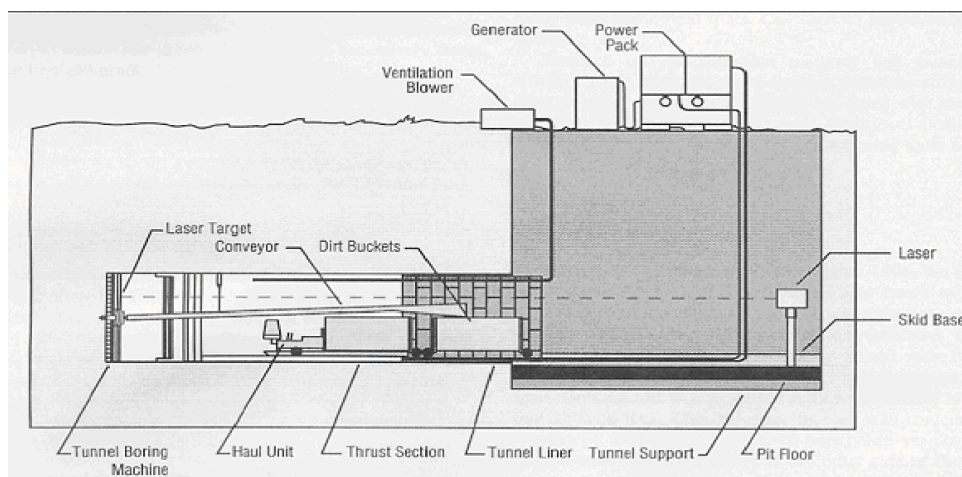


Figure 5 - Typical components of utility tunnelling method

2.3.2. Pipe jacking method

Pipe jacking is a trenchless technique for installing new underground pipelines and culverts. The essential element that distinguishes Pipe Jacking (as compared to other tunnelling methods) is that the lining of the resulting tunnel is pushed through the ground from the starting point rather than being built section-by-section just behind the excavation face or within the tail shield of a tunnel boring machine (Raymond L. Sterling, 2018). Pipe Jacking is similar to the Utility tunnelling method, with the exception that it combines the excavation and pipe installation into one step.

- Product pipe sizes 42 inches and larger.
- Limitations on length and size based on logistical considerations and safety.
- Pipe Jacking method can be identified in various forms including;
 - Conventional Pipe Jacking
 - Pilot Tube
 - Horizontal Auger Boring (Bore and Jack)
 - Microtunneling

Table 3 summarises the characteristics of Pipe Jacking method and Figure 6 demonstrates Pipe Jacking components.

Table 3 - Characteristic of pipe jacking

Characteristic of pipe jacking					
Method	Diameter Range (inches)	Typical Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Pipe Jacking	42” & large	1,600	RCP, GRP, Steel	Pressure & Gravity Pipelines	~ 1

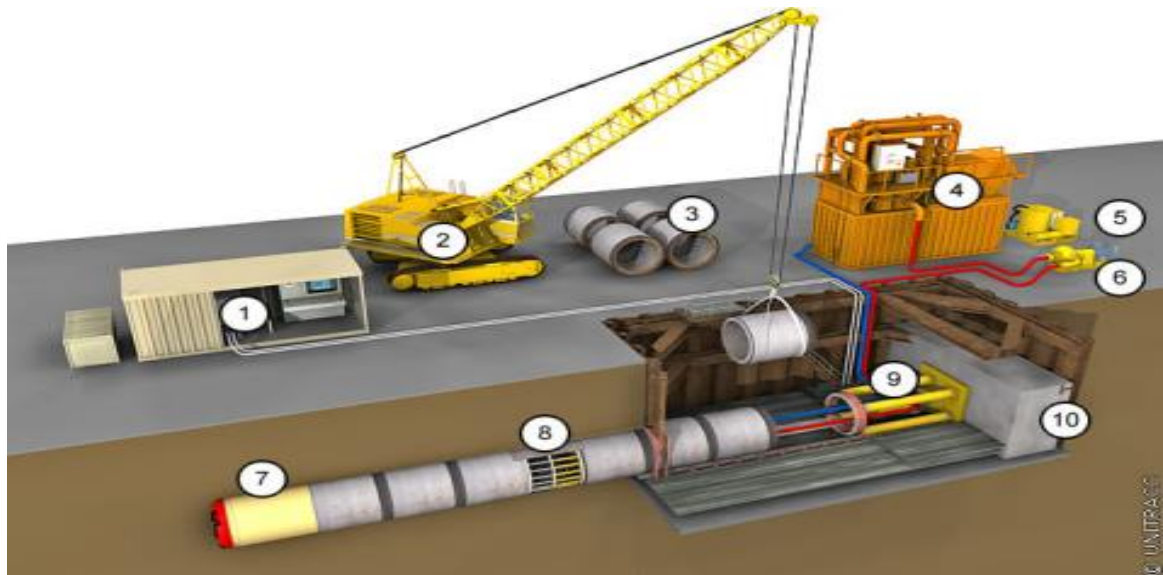


Figure 6 - Pipe jacking components

- | | |
|----------------------------|---------------------------------|
| 1. Control & steering desk | 6. Supply pump |
| 2. Crane | 7. Shield machine |
| 3. Jacking pipes | 8. Intermediate jacking station |
| 4. Separation plant | 9. Main jacking station |
| 5. Mixing plant | 10. Abutment (Thrust block) |

Figure 7 demonstrates typical components of a Pipe Jacking operation.

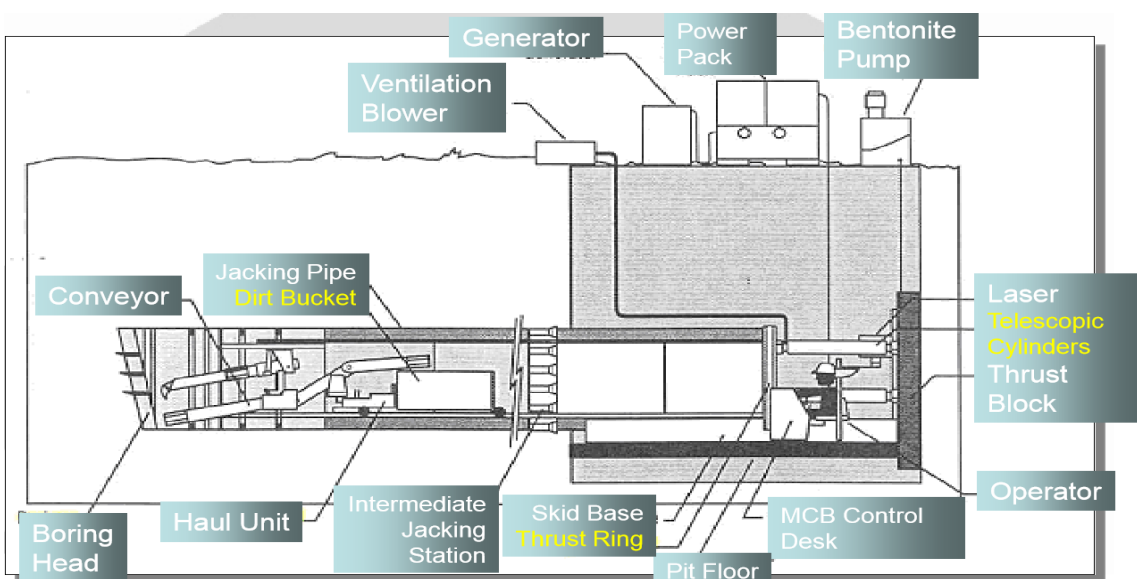


Figure 7 - Typical components of a pipe jacking operation

2.3.3. Horizontal auger boring method

Horizontal Auger Boring (HBA) method refers to the process of simultaneously jacking casing through the earth while removing the spoil inside the encasement by means of a rotating flight auger (Kendon, 2019). Horizontal auger boring is one of the oldest forms of trenchless technology and is suitable for use in a number of different applications. HBA is well suited for pipelines that begin and end on grade or have a gravity run off design as the steering requirements for these applications is quite limited. Furthermore, this method suits softer ground conditions as casing installation along with the excavation of soil minimizes the risk of seepage or soil collapse (Kendon, 2019). HBA is performed in two steps: excavation and installation of the casing pipe and Installation of carrier pipe and filling annular space with grout. This method is available with Dynamic grade control and Dynamic line and grade control. Table 4 summarises characteristics of Horizontal Auger Boring and Figure 8 demonstrates the process of Horizontal Auger Boring method.

Table 4 - Characteristic of horizontal auger boring

Characteristic of horizontal auger boring					
Method	Diameter Range (inches)	Typical Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Auger Boring	4" -60"	600'	Steel	Road crossings	1% of bore length
Auger Boring w/grade control	4" -60"	600'	Steel	Road crossings	12"
Auger Boring w/ line & grade control	4" -60"	600'	Steel	Road crossings	12"

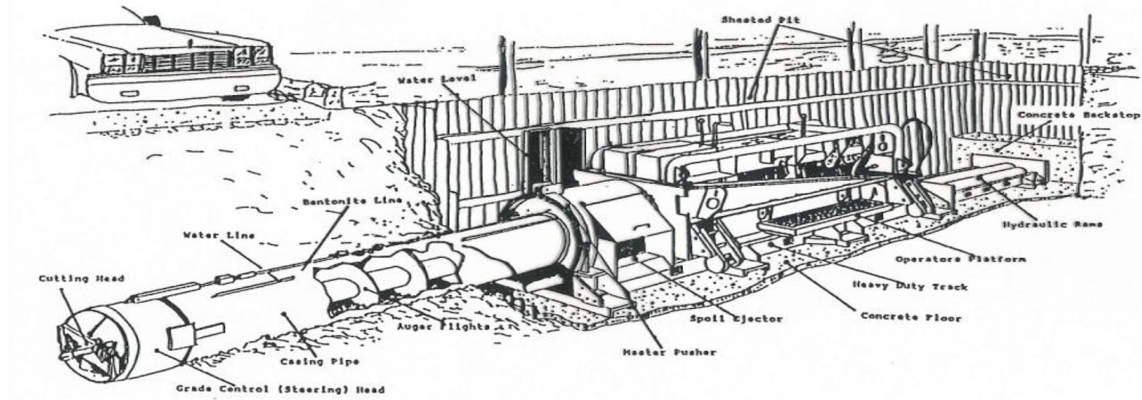


Figure 8 - Auger boring process

2.3.4. Horizontal directional drilling (HDD)

Horizontal directional drilling (HDD) is a minimal impact trenchless method of installing underground utilities such as pipe, conduit, or cables in a relatively shallow arc or radius along a prescribed underground path using a surface-launched drilling rig (Walsh, 2011). Usually performed in two (or more) steps to include; (1) Drilling of pilot hole using a steerable drill head and locator system; (2) Back reaming to increase pilot hole diameter and pullback of product pipe. This method could be used for product pipe sizes up to about 60 inches. HDD is typically used for road and river crossings. Table 5 summarises the characteristics of Horizontal Directional Drilling method and Figure 9 demonstrates the process of HDD method.

Table 5 - Characteristics of horizontal directional drilling method

Method	Diameter Range (inches)	Maximum Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Mini-HDD	4"-12"	<600	PE, PVC, DIP	Pressure Pipe & Cables	Varies
Midi-HDD	12"-24"	600-2000'	PE, Steel	Pressure Pipe	Varies
Maxi-HDD	24"-60'	2000'-6000'	Steel	Pressure Pipe	Varies

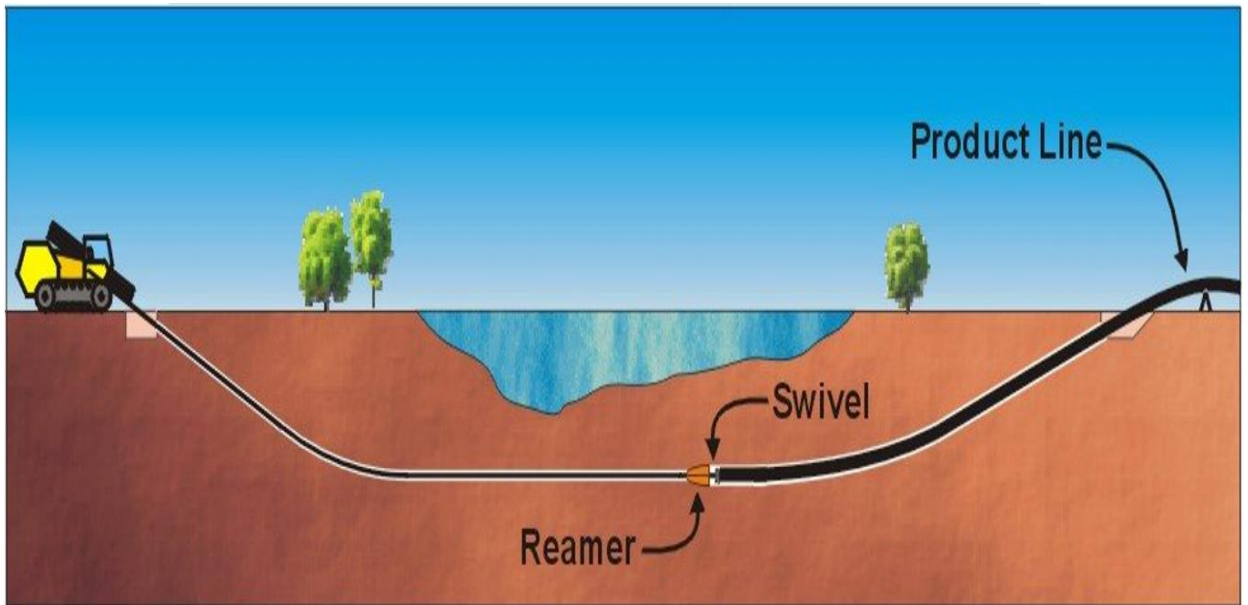


Figure 9 - Horizontal directional drilling method (HDD)

2.3.5. Pipe ramming method

Pipe ramming is a trenchless method for installation of steel pipes and casings over distances usually up to 150 feet up long and up to 55-inches in diameter, although the method can be used for much longer and larger installations. The method is the most useful for shallow installations under railways and roads, where other trenchless methods could cause surface settlement or heave. The majority of installations are horizontal, although the method can be applied for vertical installations as well (Simicevic & Sterling, 2001). This method is performed in two steps;

- Installation of the casing pipe by using an air hammer from a drive pit
- Use closed-end casing (<8" diameter)
- Use open-end casing for >8", clean spoil from casing after drive completed
- Installation of carrier pipe & filling annular space with grout

Table 6 summarises characteristics of Pipe Ramming method and Figure 10 demonstrates the process of pipe Ramming method.

Table 6 - Pipe ramming method

Method	Diameter Range (inches)	Typical Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Pipe Ramming	< 150"	250	Steel	Road crossings	Depends on setup

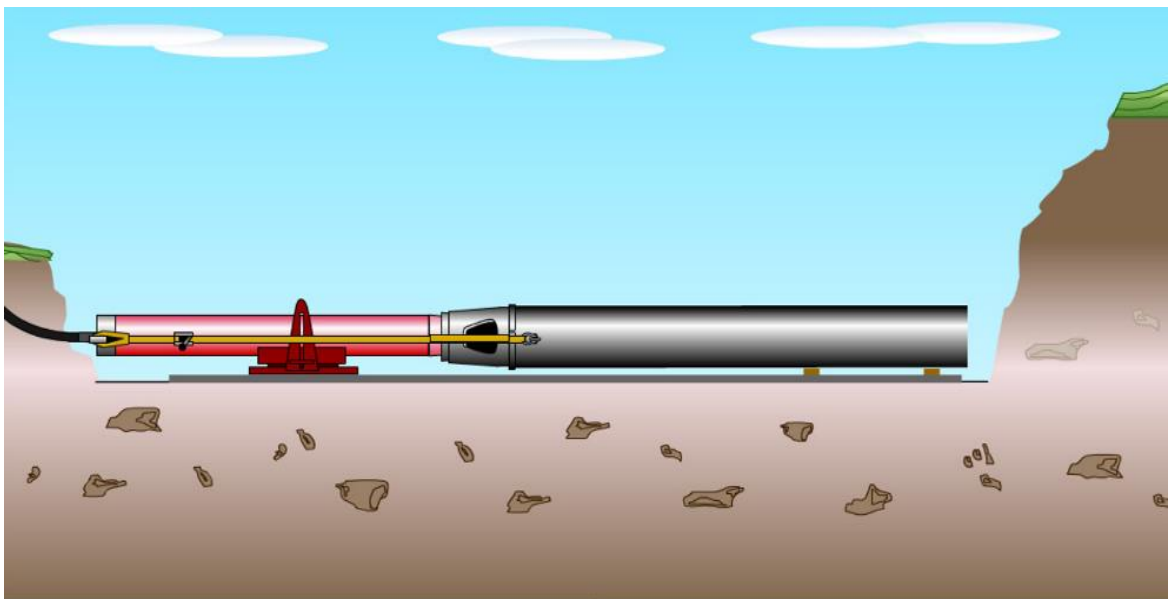


Figure 10 - Pipe ramming method

2.3.6. Microtunneling method (Gravity pipelines)

The concept of Microtunneling can be defined as “a trenchless construction method for installing pipelines with all of the following features utilised: remote-controlled from a control panel on ground surface, laser guided, pipe sections jacked simultaneously as spoil is excavated, and continuous support at the face of the excavation (Salem, Elwakil, & Hegab, 2017; Raymond L. Sterling, 2018). The differences between Microtunneling and the pipe jacking method can be demonstrated in the diameter of pipe installations. Microtunneling involves pipe installations up to 900 mm in diameter; whereas, pipe jacking involves pipe installations greater than 900 mm in diameter (Khondoker, Yi, & Bayat, 2016; PJA, 1995). This new ability of performing constructions in non-person entry diameters was given the name “Microtunneling” because it allowed a controlled tunnelling process in diameters below those permissible for person entry (Raymond L. Sterling, 2018). Also known as remote-controlled pipe jacking, Microtunneling method uses automation for processes performed by workers within the tunnel on pipe jacking, remote controlled excavation and spoil removal; and remote controlled guidance system (Salem *et al.*, 2017). Table 7 summarises the characteristics of Microtunneling method and Figure 11 demonstrates the characteristics of Microtunneling project.

Table 7 - Microtunneling method

Method	Diameter Range (inches)	Maximum Installation (feet)	Pipe Materials	Typical Applications	Accuracy (+ or -)
Microtunneling	> 12”	1,000’	RCP, GRP, VCP, DIP, Steel, PCP	Gravity Pipelines	~1”



Figure 11 - Cut-away illustration of a microtunneling project (Raymond L. Sterling, 2018)

Figure 12 & 13 demonstrate the Microtunneling boring machines.



Figure 12 - Microtunnel boring machine (MTBM)



Figure 13 - Micro tunnel boring machine (MTBM)

2.4 History of microtunneling

The developments leading to modern Microtunneling machine excavation occurred principally in Japan, Germany and the UK (Salem *et al.*, 2017). Microtunneling was developed by the Japanese in the early 1970's to replace open sewers in urban areas with underground gravity sewers. The first Microtunneling project in the U.S occurred in south Florida in 1984 (Khandeshe, Ambhore, Kawade, Jaybhaye, & Supekar, 2018). Microtunneling technology rapidly developed along with the inventions such as slurry machine, mud shield and hydro shield

2.5 Microtunneling technology

Microtunneling operations involve a complex interaction of processes that require a variety of supporting equipment and personal experience. The processes of a Microtunneling construction project include planning, collecting data and subsurface exploration, laboratory testing, preparation of geotechnical reports such as geotechnical data report (GDR) and a geotechnical baseline report (GBR), preparation of bidding documents and Microtunneling construction (Kilduff & Wilkinson, 2017). Microtunneling constructions usually include the following steps (City of Portland, Oregon, 2018):

- Excavate launching and reception shafts at opposite ends of the tunnelling drive.
- Hydraulic jacks in the launch shaft push a micro tunnel boring machine (MTBM) into the earth.
- Pipes containing slurry water transport excavated spoils to the surface.
- Retract the jacks and disconnect slurry lines and control cables.
- Lower a pipe or casing into the shaft and insert it between the jacking frame and the MTBM.
- Reconnect slurry lines and control cables and advance the MTBM another drive.
- Repeat the process until the MTBM reaches the reception shaft.
- Retrieve the MTBM and trailing equipment.

Figure 14 demonstrate the Microtunneling system layout

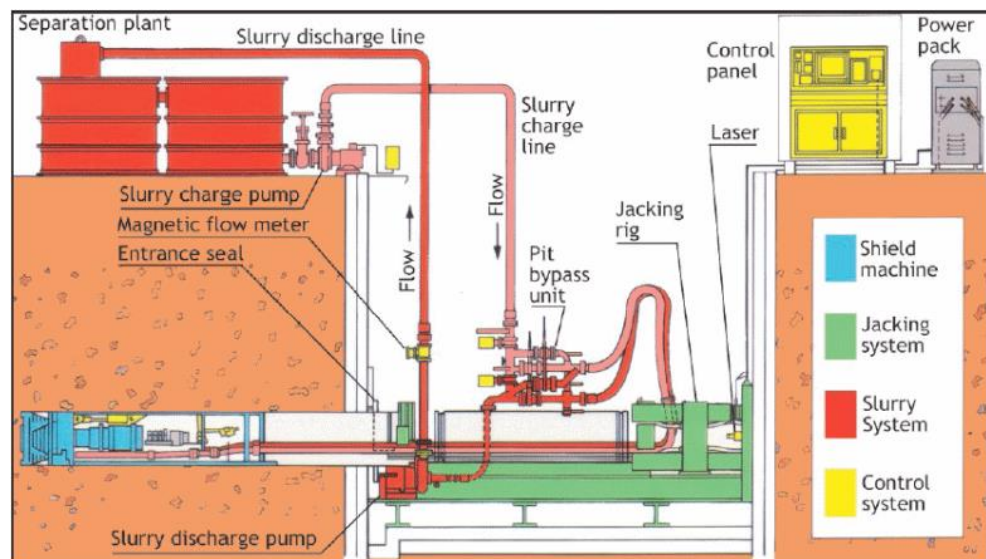


Figure 14 - Microtunneling system layout (Nicholas, 2014)

Figure 15 demonstrate the typical microtunneling site and Figure 16 demonstrate the Typical arrangement of microtunneling jacking shaft.

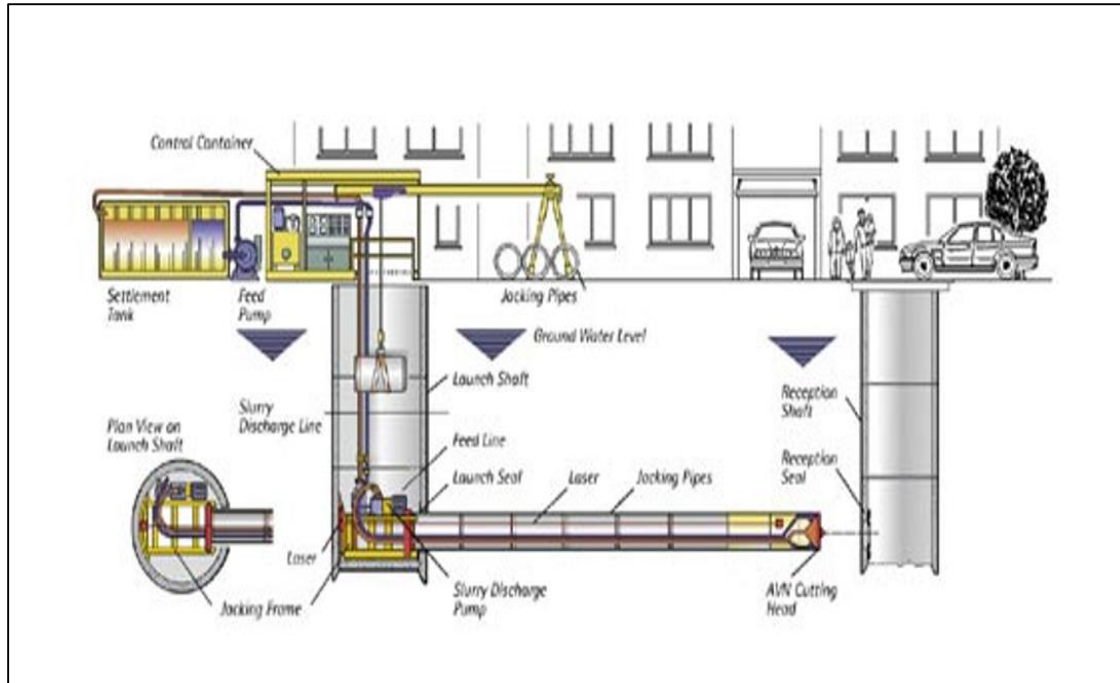


Figure 15 - Typical microtunneling site



Figure 16 -Typical arrangement of microtunneling jacking shaft

2.6 Economics of Microtunneling

Cost plays an important role in the decision-making process when undertaking a underground excavation based construction project. An economic evaluation involves the assessment of both the initial construction cost and associated benefits of each method under consideration (Hay, 2013). The goal of which is the selection of a single alternative with the greatest benefit at the lowest overall cost (Hay, 2013; Ormsby, 2009). These costs can be divided into the following categories (Ormsby, 2009).

- Direct costs;
- Indirect costs; and
- Socio-economic inconvenience costs.

It is often very difficult to use historical data to obtain average cost with acceptable variance as numerous factors come into play, such as;

- Soil type, Number of pits & Pipe diameter
- Degree of difficulty in meeting the performance requirements
- Location/country (lack of local labour and technical staff)
- Some rates include pipe cost as well (Percentage cost of pipes approximately 20)
- It may better to obtain guidance from a recent project implemented under similar condition and similar location;

It has been shown in past research that trenchless technologies have the potential to reduce costs, mainly because excavation volumes can be greatly reduced through these ‘low-dig’ or ‘no-dig’ methods. Figure 17 compares the costs involved in open-cut construction methods versus trenchless methods. Figure 18 demonstrates the cost-effective route selection method used in Microtunneling technology.

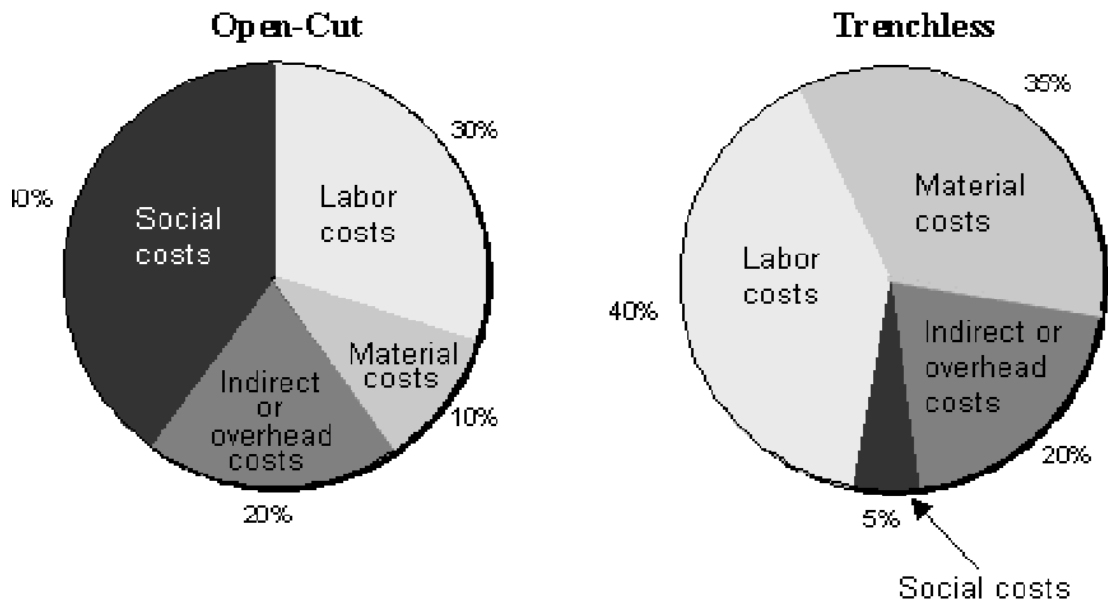


Figure 17 - Comparison of cost-breakdown for open-cut and trenchless methods.

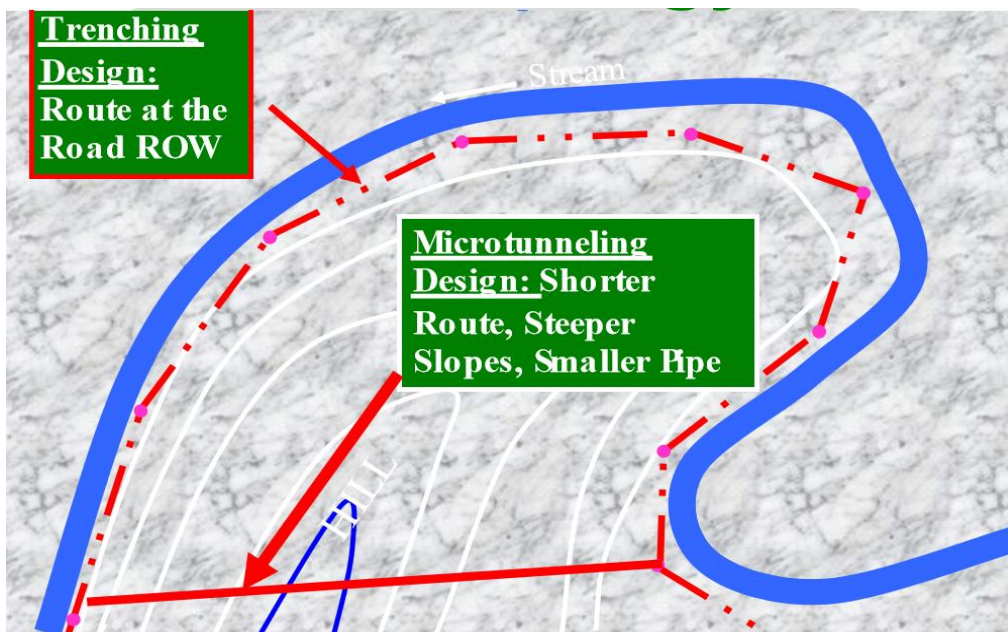


Figure 18 - Cost-effective route selection.

Table 8- Comparison of cost factors between open-cut and trenchless technology

Factor	Open-Cut	Trenchless Technology
Depth	Major	Minor
Diameter	Moderate	Moderate
Soil Conditions	Major	Moderate to Minor
Obstructions	Major	Minor
Water Table	Major	Minor
Existing Utilities	Major	Major to Moderate
Damage to Pavement	Major	Minor
Reinstatement	Major	Minor
Traffic	Major	Minor
Safety Issues	Major	Minor
Productivity	Major	Minor
Environmental Issues	Major	Minor

Table 8 describes the comparison of cost factors between open-cut and trenchless technology

2.6.1 Direct costs

Direct costs are the project specific costs of conception, development and implementation that are borne by the client organization and which are normally considered in the contract (Hay, 2013). Figure 19 demonstrates components of direct costs for underground utility projects.

Figure 19 demonstrates the cost identification for underground utility project.



Figure 19 - Cost identification for underground utility project (Woodroffe & Ariaratnam, 2008)

These include costs such as;

- Overhead,
- Construction costs (materials, equipment and labour),
- Restoration,
- Traffic control,
- Taxes,
- Insurance,
- Emergency and temporary services,
- Contingency.

2.6.2 Indirect costs

Indirect costs are the costs that are borne by the client organization that normally accrue during or after construction as a result of extraneous factors and actions performed in the project. Indirect costs include;

- Pavement service life reduction,
- Pavement damage due to excavation
- Pavement damage on alternate routes due to increased traffic

- Buried utility damage, unplanned property damage,
- Loss of parking metre and ticketing revenue, Compensation
- Loss of municipal tax revenue

2.6.3 Socio-economic inconvenience costs (External costs)

External costs are the sum of all social, environmental and economic costs, market priced and non- market priced, that accrue to third parties at any time as a result of actions promoted by the client organization. These kinds of costs could include;

- Increased travel time,
- Increased vehicle operating costs and collision rates;
- Obstruction of emergency vehicles;
- Lost business income and property value;
- Accidental injury and death;
- Air pollution and greenhouse gas emissions;
- Water and ground contamination;
- Loss of amenity and dust and dirt generation

Therefore, external cost of an excavation project could be understood in the following terms;

Social inconvenience costs

Increased vehicle travel time

Increased vehicle operating costs

Increased pedestrian travel time

Increased collision rate

Obstruction to emergency vehicles

Accidental injury and death

Property damage

Water and wastewater service interruption

Psychological and physical ailments

Economic costs

Lost business income

Lost property value due to noise

Environmental costs

Air pollutant and greenhouse gas emissions

Environmental damage and contamination

Dust and dirty pollution

Damage and lost amenity of recreational facilities

Total cost = Direct cost + Indirect cost + Social cost

Figure 20 illustrates cost components of indirect and social costs.

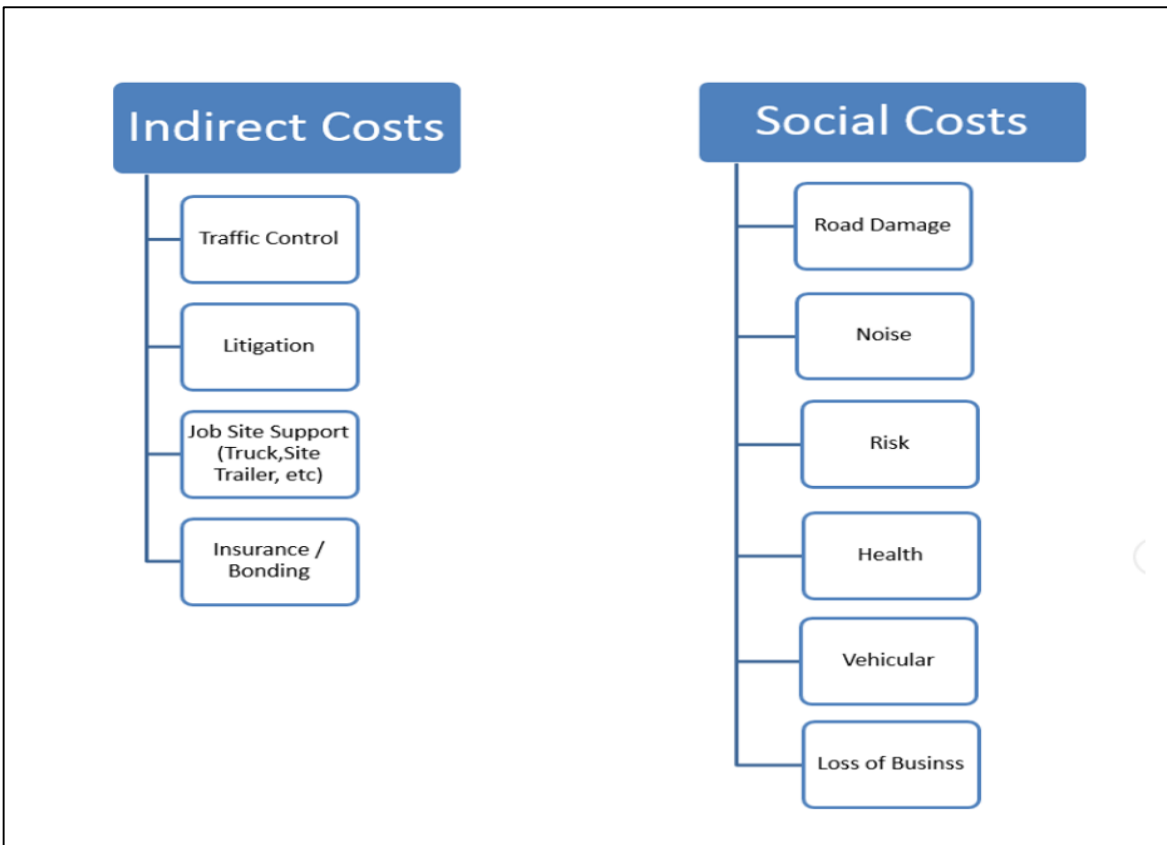


Figure 20 - Cost components of indirect and social costs

A study of the existing literature has revealed that, particularly in the area of buried municipal infrastructure renewal projects, many definitions of social costs have been proposed over the past 12 years (Allouche, Ariaratnam, & AbouRizk, 2000; Ormsby, 2009). For instance, some researchers have considered social costs to be only the costs incurred by third parties who do not engage in the contractual agreement, to include unmeasured costs such as the damage to the environment caused by air pollution, noise, vibration and also the loss of amenity and the disruption to commercial and private traffic. Other costs accrued during the lifecycle of a project are termed either direct or indirect costs to the municipality. Thus, the total cost of a project would be the sum of direct, indirect and social costs.

Working on this notion and based on the area of impact, such as traffic, economic activities, pollution and ecological/social/health related impacts, Gilchrist and Allouche (2005) further separate the social costs into four categories as demonstrated in Figure 21 and 22.

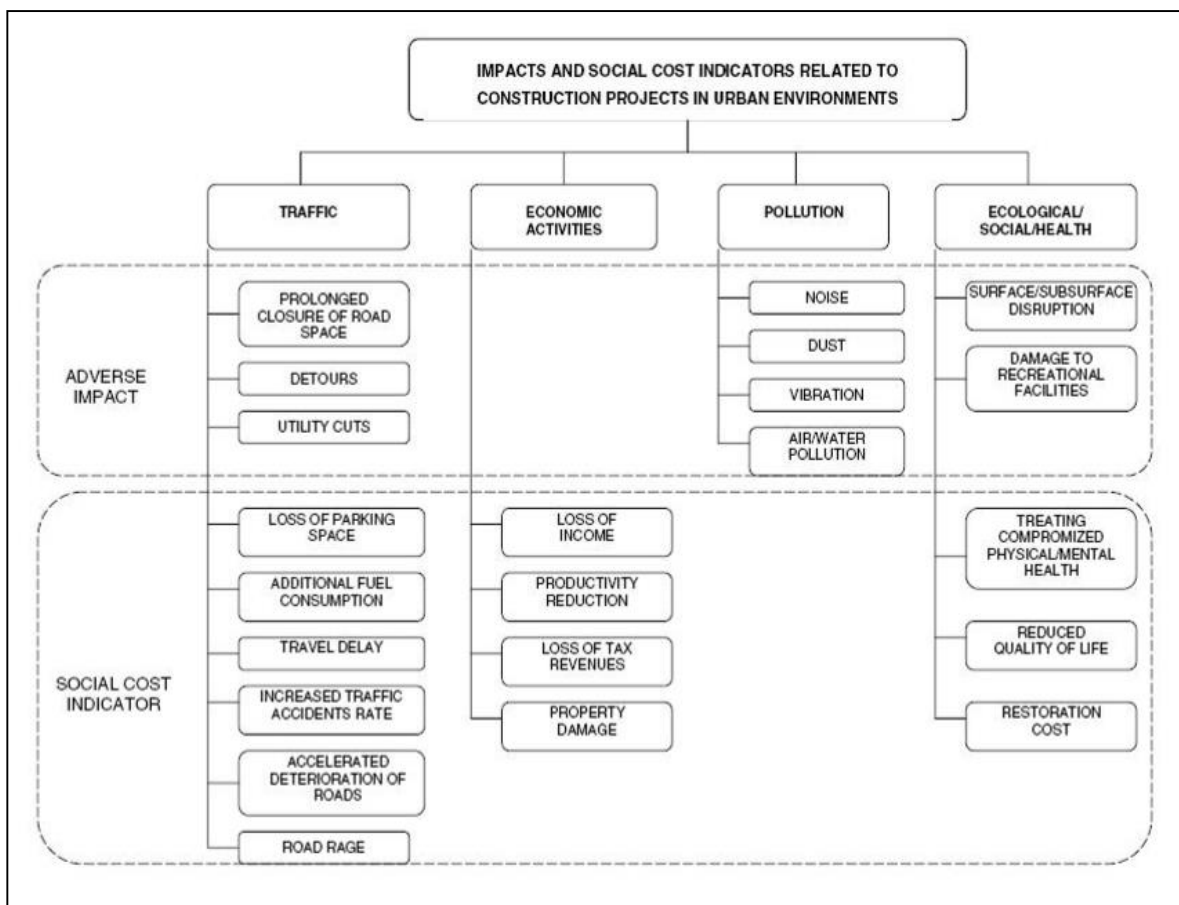


Figure 21 - Social impacts and cost indicators according to Gilchrist and Allouche (2005)

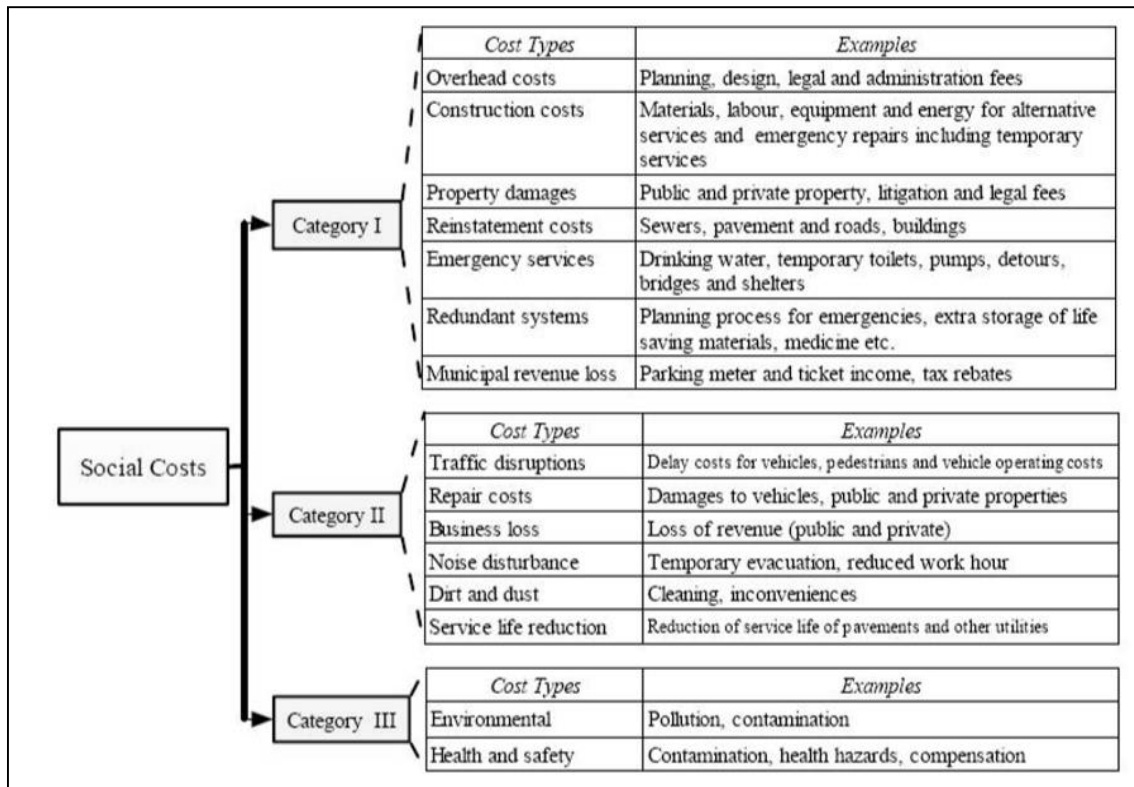


Figure 22 - Social cost categories according to Rahman, Newton and Vanier (2005)

2.7 Summary

This chapter evaluated literature in current excavation technology to discuss open-trench construction method versus trenchless technology. The chapter introduced open-trench construction methods and discussed their disadvantages to focus on trenchless technology and their advantages. The chapter then discussed key forms of trenchless technology before moving into a discussion of the concern of the present study; Microtunneling technology. Microtunneling technology was briefly discussed with the key steps involved in the process. Various forms of costs incurred in the excavation-based construction projects were discussed to establish that the total costs of trenchless technologies are significantly lower compared with the open-trench constructions.

The literature review chapter concludes that the indirect and socio-economic inconvenience costs which were often ignored resulted in costly expenses to the municipalities. Therefore, this literature review proclaims that these indirect and socio-economic inconvenience costs must form part of the total cost of a project as it assists with the successful completion of the project without expensive unforeseen costs to the municipalities.

3 RESEARCH METHODOLOGY

The purpose of this chapter is to clarify the methodological framework used to conduct this research. This chapter outlines the steps undertaken to design, plan and implement this research. It will cover the methodological approach and methods used to select and design research instruments; collect and analyse data. Because the background and context of the subject matter are influences on all aspects of this study, they are discussed. Finally, the ethical considerations for the undertaking of the study are outlined as a conclusion to the chapter.

3.1 Research design

A case study approach was implemented in this research and priority was given to identify different forms of costs incurred in Microtunneling construction projects in Colombo, Sri Lanka to establish a conceptual framework for critical analysis of Microtunneling in Sri Lanka. Reviewing relevant literature on buried infrastructure installation, maintenance and rehabilitation projects helped limit the scope of the research, to identify theories, frameworks, models and findings by other researchers.

3.2 Nature of the inquiry

The case study, according to Creswel (1998), explores a bounded system or case over time "through detailed, in-depth data collection involving multiple sources of information rich in context". In investigating complex situation, such as construction projects, case study approach has been proven reliable to capture the rich information for the purpose of the investigation. Case study method has been regarded allowing investigators to retain the holistic and meaningful characteristics of real-life events (Sutrisna & Abbott, 2007).

Case study approach has been widely used in the research involving buried municipal infrastructure construction projects for various different research purposes. The researcher reviewed the methodologies implemented in several past researchers to identify substantial number of studies that have used the case study methodology. Table 9 summarises methodologies been used in the aforementioned past studies.

Table 9 - Methodologies used in research involving buried municipal infrastructure construction projects

Author/s	Study	Methodology
Ormsby (2009)	A framework for estimating the total cost of buried municipal infrastructure renewal projects	Case study
Hashemi (2008)	Construction cost of underground infrastructure renewal: A comparison of traditional open-cut and pipe bursting technology	Literature analysis, Survey, Regression analysis Case study
Hafez, Aziz, & Attia (2015)	Exploring critical factors affecting upon Microtunneling equipment productivity	Questionnaire/ factor analysis
Hay (2013)	A comparative study of trenchless technology versus traditional open trenching for the replacement of ageing potable water pipelines	Survey
Ranasinghe (2015)	Implications of Microtunneling on wastewater pipeline constructions: A case study from Muscat wastewater system: Al Khuwair project	Case study
Nicholas (2014)	Microtunneling in India	Case study
Gottipati (2011)	Pilot tube Microtunneling: Profile of an emerging industry	Survey
Salem, Elwakil, & Hegab (2017)	Risk level problems affecting Microtunneling projects Installation	Cluster analysis of survey and literature data
Khandeshe, Ambhore, Kawade, Jaybhaye, & Supekar (2018)	Trenchless technology: Microtunneling	Case study

(Chung, Abraham, & Gokhale (2004)	Decision support system for Microtunneling applications	Case study Decision support system (DSS)
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Even though Microtunneling technology is highly sophisticated and regularly used in buried infrastructure related projects in developed countries, its use in Sri Lanka is recent and limited. Furthermore, research conducted on Microtunneling projects in the Sri Lankan context are very limited. Descriptive case studies are appropriate in this kind of setting where only a little research has been conducted (Sutrisna & Abbott, 2007).

Therefore, the case study method was chosen to examine Microtunneling construction projects implemented in Sri Lanka. The aim of this research was to establish the cost parameters of Microtunneling and to propose a framework for critical analysis of Microtunneling in Sri Lanka. Accordingly, the research problem could be developed as “what are the costs incurred in Microtunneling projects in Sri Lanka and what are the factors affecting different types of costs?” Accordingly, the researcher implemented case study methodology to collect quantitative as well as qualitative data from aforementioned Microtunneling construction projects implemented in Sri Lanka.

3.3 Data collection

Literature review was undertaken initially to help establish a rationale for the research questions and to ascertain the extent and depth of existing knowledge on cost parameters of Microtunneling technology. Primary data were collected in the form of semi-structured interviews and document analysis.

Semi-structured interviews

Semi-structured interviews of varying durations (30 minutes to 1.5 hours) were conducted with key decision makers and engineers involved in the Microtunneling construction projects implemented in Sri Lanka. The focuses of the interviews were to obtain first hand data regarding;

- (1) Nature of the constructions involved.
- (2) Processes and procedures during the planning stage

- (3) Processes and procedures during the construction stage
- (4) Processes and procedures after the completion of the project
- (5) Identification of direct costs incurred
- (6) Identification of indirect costs involved
- (7) Measures taken to establish social and environment cost

Document analysis

Documents relevant to the cases under investigation were collated, reviewed and analysed to gather data pertinent to aforementioned aspects.

3.4 Data analysis

Field notes that were taken during the interviews were organised with reflective comments and further questions. The field notes were transcribed into Microsoft Word and sought for emerging codes, patterns and themes. Different types of cost components were analysed using the methodology proposed by (Ormsby, 2009). The strong themes emerged through the analysis of qualitative data gathered through the interviews were linked with the cost component analysis to establish cost parameters of Microtunneling projects implemented in Sri Lanka.

3.5 Ethical considerations

Construction organisations' confidential data were not disclosed in the case studies. Interview participants were ensured of their anonymity when they wanted. The researcher adhered to the ethical conducts when obtaining written data from the participants.

3.6 Summary

This chapter has presented research approach and research process used in this research study. The next chapter analyses and discusses the findings from the study in detail.

4 CASE STUDY OF MICROTUNNELING IN COLOMBO CITY

The case study focused on three aspects

1. Technology used
2. Cost Comparison
3. Parameters for analysis for applications in Sri Lanka.

4.1 History and projects

1. The Asian Development Bank (ADB) approved a loan to upgrade the services of the Wastewater Division of the Colombo Municipal Council, The executive agency is Ministry of Local Government & Provincial Councils and the implementing agency is the Colombo Municipal Council.

The expected impact of the project included an improved urban environment and better public health for residents in the Colombo City and suburbs connected to the sewer system through improved marine and inland water quality and a resulting improvement in hygiene and sanitary conditions. The expected project outcome was improved wastewater management services provided to residents within the project area through upgraded sewerage infrastructure and enhanced institutional and operational capacity of the service provider

2. In the Colombo basin there are five main storm water drainage outlets, namely Ambatale, North lock, Mutwal tunnel, Dehiwala and Wellawatta. The conveyance capacity of the outfalls and main canals are found to be inadequate with the increasing intensity of the peak flows, to cope with the extreme flood peaks. Acquisition of land to enhance retention capacity had been restricted by rapid urbanisation and development of infrastructure Thus, finding new discharge outlets using tunnelling techniques in the densely populated city of Colombo is considered to be the most feasible solution, in order to minimize the flood impacts.

One of the Storm Water projects funded by World Bank, proposed to reduce flooding in Colombo is commenced to drain rain water accumulated from Thummulla area. The proposed Torrington Tunnel which forms part of the macro drainage system that integrates with the micro drainage system within the catchment areas. The storm water is to be diverted into the sea through the proposed network of tunnels, under this scheme. One of the primary objectives is to transfer into the Tunnel network, most of the storm water that would have flowed into the main canal system.

4.2 Microtunneling projects in Sri Lanka

4.2.1 Case Study 01

Rehabilitation and Upsize 10 kilometres of identified problematic and critical sewers of the Colombo sewerage system within the Colombo Municipal Council area under ADB funds.

Figure 23 illustrates the Schematic Diagram of the Colombo Sewer System

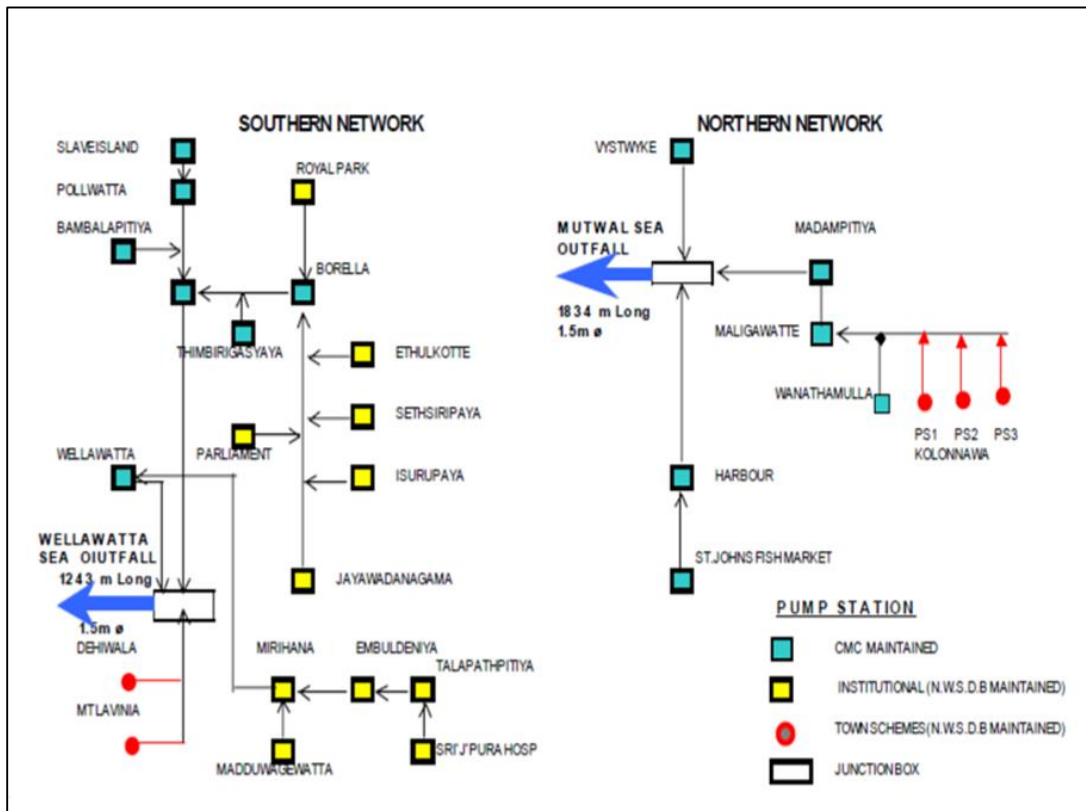


Figure 23 Schematic Diagram of the Colombo Sewer System

Figure 24 illustrates the Sewer System of Colombo.



Figure 24 Sewer System of Colombo

4.2.2 Case Study 02

Design and building of Torrington Tunnel within the Colombo Municipal Council area under World Bank funds.

The catchment areas to be drained and the proposed Torrington Tunnel network are given below. Figure 25 illustrates the proposed site layout of the Torrington tunnel and Figure 26 illustrates the sub catchment area map Torrington tunnel.



Figure 25 - Sub catchment area map Torrington tunnel

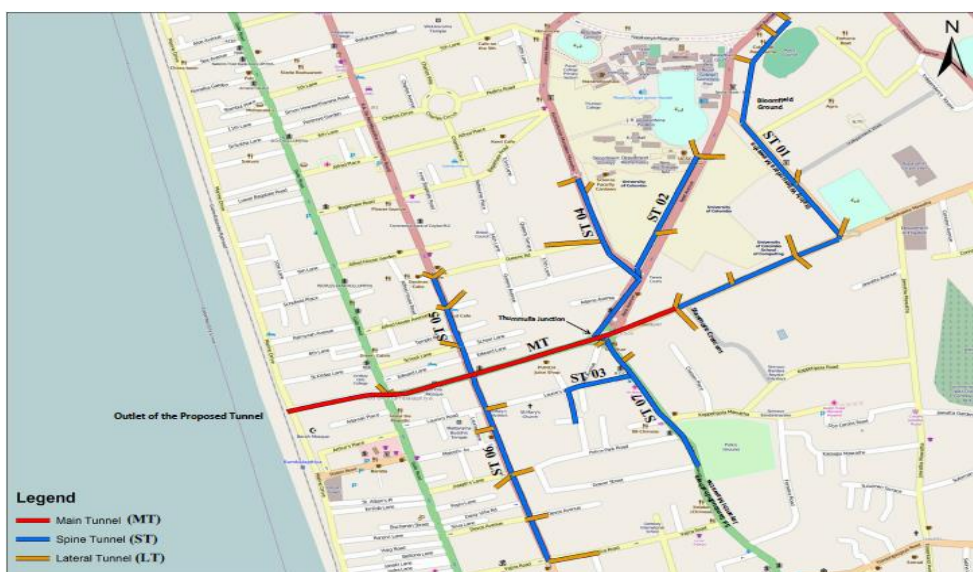


Figure 26 - Proposed site layout of the Torrington tunnel

4.3 Technology used

Rehabilitation and Upsize 10 kilometres of identified problematic and critical sewers of the Colombo sewerage system within the Colombo Municipal Council area under ADB funds.

The Colombo city is overcrowded and streets are narrow and full of traffic even in the night. Under these circumstances it is really challenging to rehabilitate a 100-year-old sewer system. Therefore, it was more appropriate to adopt in situ No Dig techniques for the rehabilitation of this sewer system.

Figure 27 illustrates the Typical Microtunneling site.

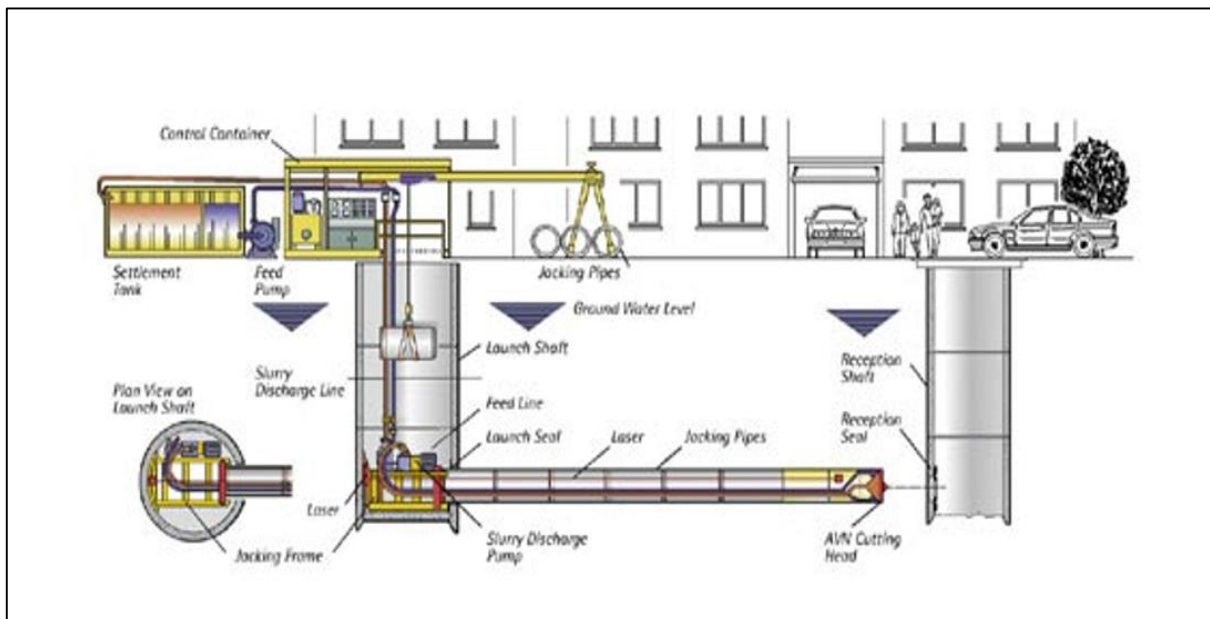


Figure 27 - Typical Microtunneling site

For gravity sewers- The Microtunneling technique used for the installation of the trenchless pipeline is pipe jacking and machine used is MTBM.

For Force mains- The Microtunneling technique used for the installation of the trenchless pipeline is Horizontal Directional Drilling (HDD) and machine used is MTBM.

Figure 28 illustrates the Route Plan of Wanathamulla Forcemain Area.

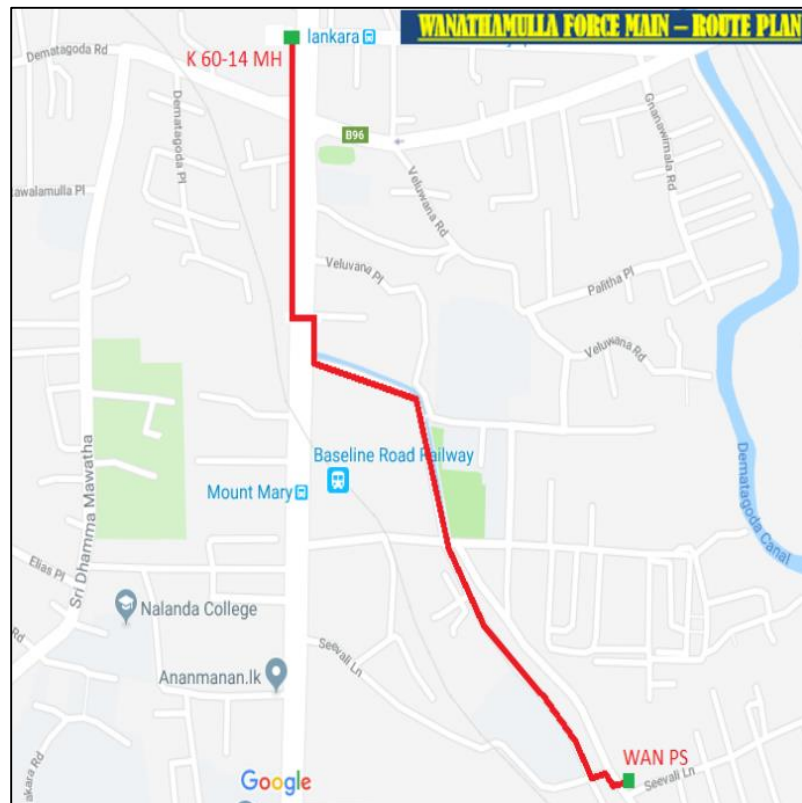


Figure 28 - Route Plan of Wanathamulla Forcemain Area

Pipe jacking technology is used for bellow mentioned conditions,

- Most suitable for gravity sewers.
- Applied for either soft or hard ground.
- Suitable for a range of soil, from medium to coarse grain sand clayey gravels.
- Suitable for rigid pipes & need to be sharp alignment.
- Minimum working space requirement.
- Suitable for either larger or smaller diameter pipes.

Figure 29 illustrates the Installation of Jacking Machine and Figure 30 illustrates the operation of the Jacking machine.



Figure 29 Installation of Jacking Machine



Figure 30 Jacking machine is in operation

HDD technology is used for bellow mentioned conditions,

- Most suitable for force main sewers.
- Suitable for flexible pipes & no need to be sharp alignment.
- More working space requirement.
- Applied for either soft or hard ground.
- can be utilized with various pipe materials such as PVC, polyethylene, polypropylene, ductile iron,
- Suitable for a variety of soil conditions including clay, silt, sand, and rock

Figure 31 illustrates the Carryover of TBM from one place to another.



Figure 31 -Carryover of TBM from one place to another

Figure 32 illustrates the Preparation of driving shaft at Manhole No.18 and Figure 33 illustrates the Checking the pipe alignment.



Figure 32 - Preparation of driving shaft at Manhole No.18



Figure 33 - Checking the pipe alignment

Design and building of Torrington Tunnel within the Colombo Municipal Council area under World Bank funds.

The Microtunneling technique used for the installation of the trenchless pipeline is pipe jacking and machine used is Horizontal Jacking and Boring Unit.

The pipe jacking technique, as described above is to be used for the installation of a trenchless pipeline along Bauddhaloka Mawatha from Stanmore Crescent to the sea outfall. This section of the tunnel runs through the Thummulla junction (a large multi-exit round-about), Duplication Road, Galle Road and Marine Drive. (Micro Tunnelling concept paper, 2016)

The required increase in capacity of the system has been modelled and verified using Mike Urban software, accredited for micro drainage under the World Bank funded projects, in Sri Lanka. Hydraulic modelling for the extended micro drainage networks of the CMC area has been developed, by the Sri Lanka Land Reclamation & Development Corporation (SLLRDC) and Colombo Municipal Council (CMC), with the assistance of the COWI Consultant, using the said software.

The initial phase of the feasibility studies stipulated a main tunnel internal diameter of 2.5m. Subsequently, the addition of flows from certain critical locations of the bordering catchment areas demanded an enhanced internal diameter of 3.0m. It has been proposed that this section of the tunnel be constructed using pipe jacking. It has been found appropriate that the previously intended cut-and-cover tunnel sections, to be integrated into this scheme, be constructed using the auger boring technique. The latter technique, adopted in place of the previously intended cut-and-cover tunnel sections will save a considerable amount of time and money, on the overall scheme. Rehabilitation of the surface drains to channel water into the auger bored tunnels will be undertaken by the CMC, at the earliest opportunity, in order to obtain the optimum use of the Torrington Tunnel network.

The output and model conceptualization details are given below in Figures 34 and 35.



Figure 34 - Model conceptualisation

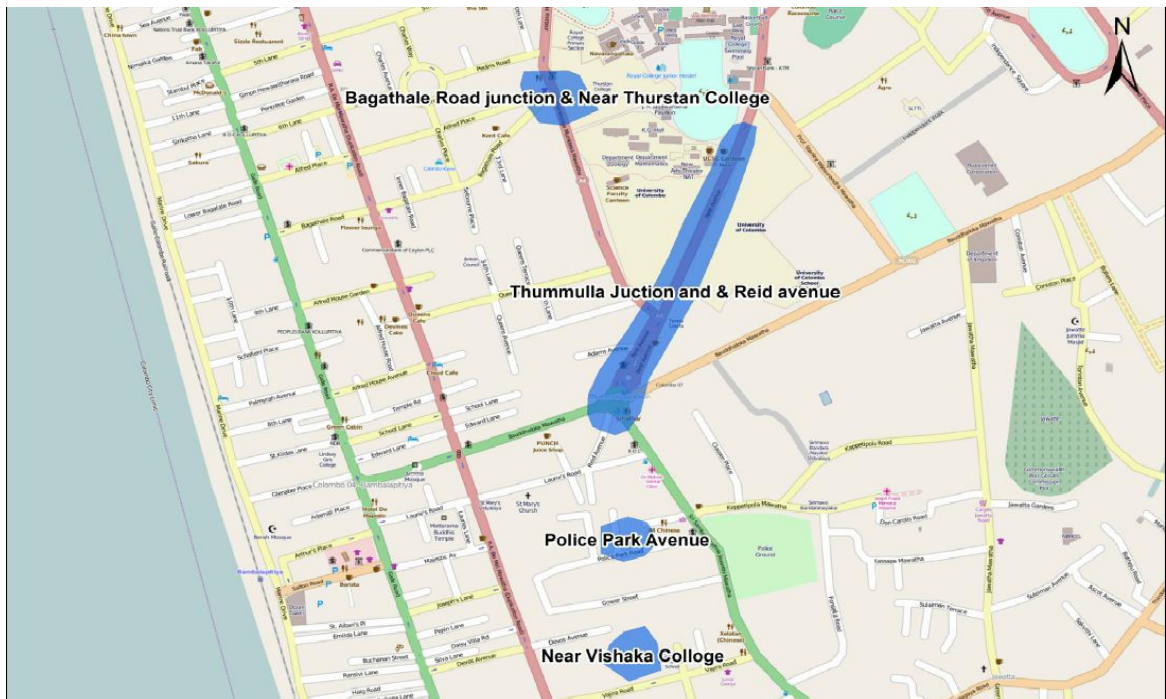


Figure 35 - Model conceptualisation

4.4 Methodology

Rehabilitation and Upsize 10 kilometres of identified problematic and critical sewers of the Colombo sewerage system within the Colombo Municipal Council area under ADB funds.

4.4.1 Pipe jacking Methodology

Figure 36 illustrates the setting-up of guide rails and jacking rig and Figure 37 illustrates setting up of TBM Shield.



Figure 36 - setting-up of guide rails and jacking rig

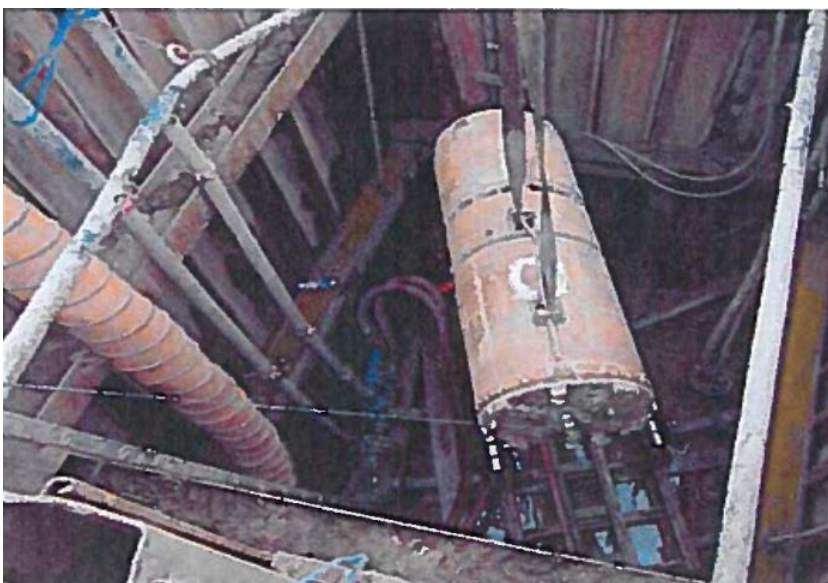


Figure 37 - setting up of TBM Shield

Two types of methods have been used in this project

- Pressurized Slurry Micro-Tunneling (PSMT)
- Pilot Tube Micro-Tunneling (PTMT)

Pressurized Slurry Micro-Tunneling (PSMT)

MTBM connected with an adopter trailed by a string of high strength product pipe segments either vitrified clay or reinforced concrete will be jacked in to the ground whilst maintaining the precise line and grade.

Figure 38 illustrates the Jacking of TBM Shield and Figure 39 illustrates the Jacking of product pipe.



Figure 38 - Jacking of TBM Shield



Figure 39 - Jacking of product pipe

Pilot Tube Micro-Tunneling (PTMT)

In the first stage, a slanted-face lead steering head trailed by guiding pipe segments will be installed to the precise line and grade. In the second stage, a soil expansion/ displacing double-coned lead trailed by high strength product pipe segments will be jacked into the ground.

Figure 40 illustrates the operation of VC pipe Jacking and Figure 41 illustrates the Jacking of steering head and pilot tubes from jacking pit.



Figure 40 - VC pipe Jacking in operation



Figure 41 - Jacking of steering head and pilot tubes from jacking pit

4.4.2 HDD Methodology

- First, a small diameter pilot hole is drilled along a directional path from one surface point to another.

Figure 42 illustrates the Pilot Hole Drilling.

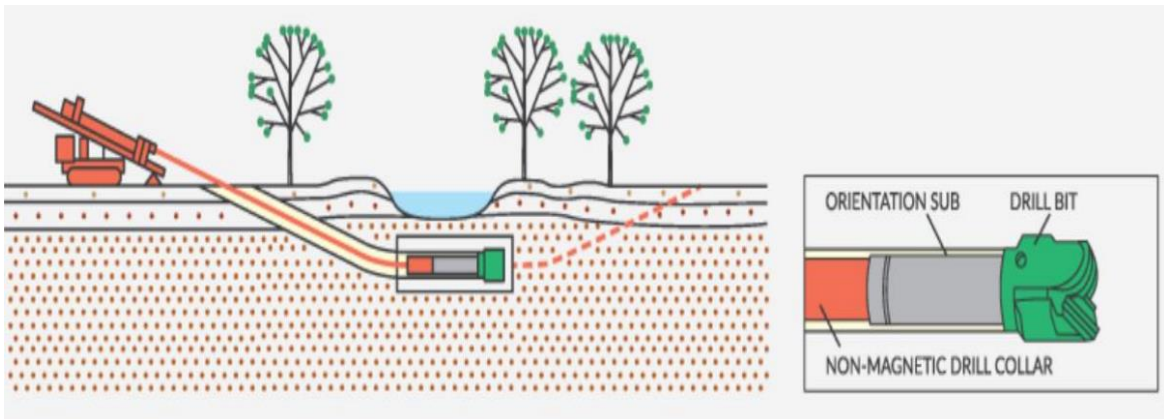


Figure 42 - Pilot Hole Drilling

- Next, the bore created during pilot hole drilling is enlarged to a diameter that will facilitate installation of the desired pipeline.

Figure 43 illustrates the Reaming.

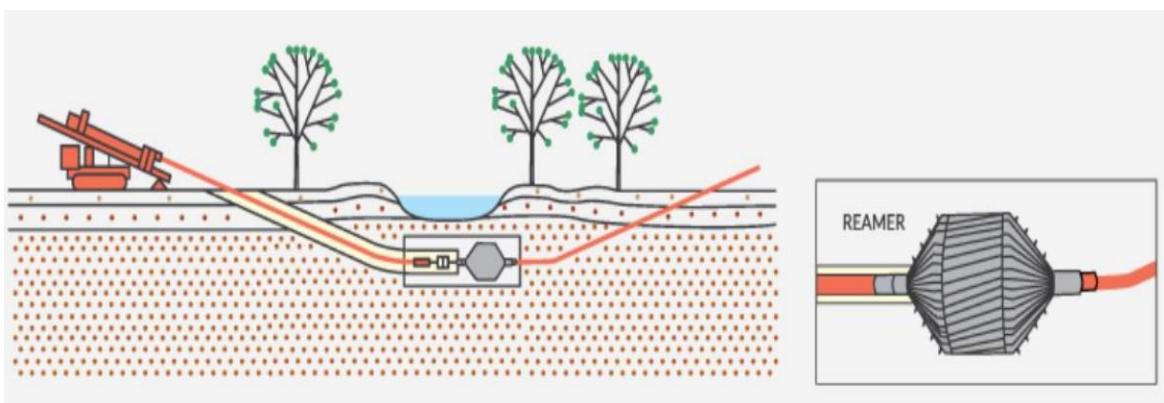


Figure 43 – Reaming

- Upon completion of the reaming process, the HDPE pipe shall be pulled in and installed inside the HDD borehole. This process is done by connecting a pulling head to the leading end of the HDPE pipe and attaching the pulling head to the pullback assembly.

The pullback assembly, together with the HDPE pipe, shall then be pulled in the HDD borehole by the HDD Rig.

The process is completed once the pullback assembly together with the HDPE pipeline reaches at the entry pit. The pullback assembly shall be dismantled from the HDPE pipe for pipe tie-in or other works to follow.

Figure 44 illustrates the Pipe Pull Back.

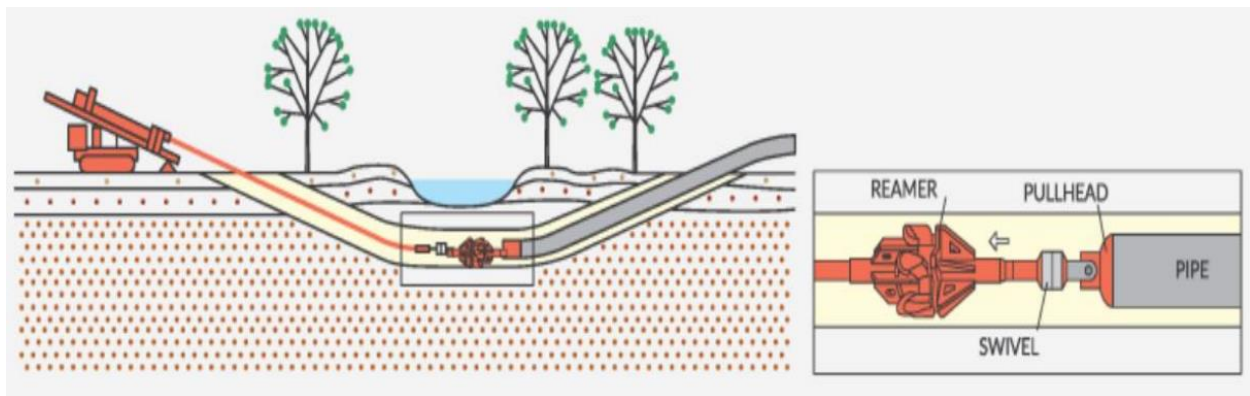


Figure 44 – Pipe Pull Back

Design and building of Torrington Tunnel within the Colombo Municipal Council area under World Bank funds.

4.4.3 Horizontal Pipe jacking Methodology

- Equipment installation
- Safety ladder installation
- Track installation
- Back wall installation
- Water stop ring installation
- Pipe Jacking set-up
- Pipe assemble and launching
- Tunnel Excavation and bentonite injection
- Tunnel finish
- Pipe jacking machine head out of the hole
- Facility disassembly

Figure 45 illustrates the Jacking alignment of Torrington Tunnel.



Figure 45 Jacking alignment of Torrington Tunnel

Figure 46 illustrates the General layout of Torrington Tunnel.



Figure 46 - General layout of Torrington Tunnel

4.5 Economic analysis

It is common practice, in densely populated and heavily built-up cities such as Colombo, for storm water to be channelled either through cut-and-cover or bored tunnels. Any open-cut construction technique would be costly and time consuming, owing to the following factors that are insurmountable, in some situations.

- Social/environmental issues.
- Service disruptions.
- Costly and time-consuming traffic management requirements.
- Very slow construction progress, particularly during inclement weather periods.
- Higher health and safety hazards to the operators and public.
- Denial and inconvenience of access to residential and business premises.
- Temporary or permanent utility diversions.
- Dewatering and water disposal issues and consequent structure movements.
- Re-instatement of affected road furniture and signage.
- Higher surface settlements, causing damage to structures.
- Backfilling and re-instatement.
- Disposal of considerable quantities of excavated material (the volume of excavated material from an equivalent cut-and-cover tunnel would be approximately fifteen times more than for the proposed tunnel).
- Much higher material, labour and plant usage.
- Physical constraints (restricted access, immovable obstacles etc.).
- Noise, vibrations and dust pollution.

For cut-and-cover construction technique to be feasible, the alignment must ideally follow an existing roadway that ought to be wide enough to maintain an uninterrupted two-way traffic flow, to permit material handling and transport. Very often, a complete road closure during construction would be essential, as many congested traffic routes would not be able to sacrifice sufficient space for construction activities, while maintaining traffic movement.

In view of the aforementioned unfavourable factors, it has been decided to examine the Microtunneling (trenchless) options, as the most suitable, in preference to cut-and-cover

techniques. This technique will eliminate or minimise the above problems and offer tremendous time and cost savings.

The utilities information has been obtained, in collaboration with the following organisations. Sufficient clearance to the utility services has been provided during the selection of the final tunnel alignments.

- National Water Supply and Drainage Board (NWSDB)
- Ceylon Electricity Board (CEB)
- Sri Lanka Telecom (SLT)
- Dialog Cable TV Company
- Colombo Municipal Council (CMC)

The input data for modelling of the sewerage system has been obtained from the records maintained by the CMC, since the sewer system is owned and maintained by them.

Several probable alignments have been examined, in order to choose the best one that offers cost and time savings. Consideration has been given to the safety of the existing buildings, mature/established trees and utility services that are within the corridor of influence of the proposed micro tunnel.

The project area is located, in close proximity to the coast and consequently, the vertical alignment is constrained by the Mean Sea Level (MSL) and the surface contours. The tunnel invert level had a lower limit governed by the MSL and the crest level by the cover requirements. Thus, the tunnel diameters and vertical alignments, apart from hydraulic requirements were chosen with consideration given to the said limitations. Lining up of the horizontal alignment away from the buildings, services and mature plants was the main criterion used in selecting the proposed alignment. This primarily prevented the unnecessary costs and delays in dealing with land acquisitions, protective work during construction and service diversions, claims for probable service disruptions and property damage.

The optimum diameter for the 1100-metre-long (approximate) main Torrington Tunnel, as formulated through hydraulic modelling is 3.0m. In addition, approximately 3890 metres of

spine tunnels with diameter varying from 0.6m to 2.0m have been proposed, under this project. These spine tunnels, effectively replace the costly and time-consuming cut-and-cover tunnels.

Channelling of flows from the shallow trenches from the catchment areas, into the proposed spine tunnels was through the lateral tunnels of diameters varying from 0.6m to 1.2m between manholes, located at strategic positions. It has been proposed that such short connections also be carried out using auger boring or similar Microtunneling methods.

The connection to the main tunnel would be directly into the main tunnel or using an off-set or tunnel aligned manholes. The shafts and lateral connections that involve contractor's working space have been located at positions of least traffic movement. Where it is necessary to deviate from a straight horizontal alignment, to ease build ability or provide clearance from major obstacles or sensitive structures, curved alignments have been introduced.

The details of the general layout are given in above diagrams

The sea-outfall, adjacent to the twin-track railway line would be approximately 50 metres in length.

The hydraulic and hydrodynamic effects at the sea out-falls have also been studied, in collaboration with the Department of Coast Conservation and Coastal Resources Management, to eliminate the possible sand-bar formation and any adverse consequences. This ensures sustainability and maintenance aspects.

Since the outlet is a sea outfall, the tide level has an influence on the discharge through the tunnel. Therefore, a separate analysis was carried out by comparing the discharge through the tunnel having different time lags to the applied time series data of tide levels. Although minor fluctuations in discharge have been observed, the effective discharge through the tunnel was acceptable.

Historical information and a few readily available boreholes, in close proximity to the proposed tunnel alignment have been used, in conjunction with experience of the CMC and SLLRDC, to assess the ground conditions for Microtunneling, tentatively, subject to more

accurate borehole information becoming available, on completion of the exploratory work, currently in progress.

The following durability and performance requirements will be written into the Bidding Documents, under preparation.

- Appropriate plant, equipment, material and skills shall be selected.
- Water conveying and retaining structures shall be designed for self-cleansing.
- All finished surfaces of structures (concrete and steel) that are prone to abrasion and chemical attack shall be protected for durability and performance. Screening of undesirable grit and debris shall form part of the design.
- Clear guidelines shall be set out for tunnel inspection and maintenance of man-entry tunnels, non-man-entry tunnels and confined spaces. This should also include access requirements, inspection frequency, appropriate equipment, contingency measures, rescue facilities and recording/reporting methods.
- Provision must be made for flow diversions for inspection (usually during dry weather periods).
- An integrated, real time flow monitoring and data logging system, remotely accessible shall be introduced.

4.6 Proposed construction sequence

The construction sequence is subject to the prospective Design and Build Contractors choice of plant/equipment, material and labour inputs. Therefore, specific sizes of the components and a rigid construction sequence cannot be laid down.

However, the following sequence of construction has been proposed, in order to give priority to flood mitigation in critical areas. It is to be noted that the Spine Tunnels have been numbered sequentially, following the said order of priority.

If open cut method was selected the following consequences would have been faced

- Materials delivered and stored in the right of way
- Pavement cut to trench width using a pavement saw
- Trenches dug to the curb to install laterals, which connect the individual properties to the sewer mains
- Pavement and material removed by large equipment and loaded into dump trucks
- Pipe installed in trench
- Trench backfilled with sand or gravel and compacted
- Temporary pavement installed as work progresses forward
- Curb and sidewalk repaired or replaced if necessary or specified in plans
- Trenches permanently paved when there is enough distance between the paving crew and the pipe crew
- Impacted planting, streets cleaned, and other remaining restoration completed as paving is finished

By selecting Microtunneling methodology for the above project the following benefits were created

- Reduced disruption of the community.
- Reduced liability for personal injury and property damage.
- Increased service life and asset value for the utility owner.
- Increased worker safety.
- Reduced restoration costs.
- Precise installation.
- Faster rate of progress than conventional tunneling
- Minimum interruption to traffic
- High quality rehabilitated sewer
- Time saving, minimum earth disturbances
- Fifty years of design life, easy ground water handling

4.7 Cost effectiveness

Microtunneling is economically competitive with direct burial when depths exceed five meter due to the costs of deep trench excavation and trench support. This method is cost effective when faced with unstable soil conditions and work below the groundwater level. These conditions increase the risk of surface settlement during a direct burial or conventional tunnel installation. These conditions also increase liability for all parties with regard to property damage and personal injury. Microtunneling is the safest tunnelling option when faced with these conditions, because workers and the public are not directly exposed to hazardous conditions.

- The equipment is noiseless and no disturbance to public
- Both preparation and post micro-tunneling work can take a few weeks to complete.
- Shafts are usually too large to cover safely with steel plates during non-construction hours. Site fencing was done to secure these areas during non-construction hours.
- No road closures and lane restrictions.
- Construction of Jacking shaft took about six months.
- No work hour restriction and pipe jacking only took three weeks
- Reduced over pumping costs
- Reduced costs for traffic regulation
- Less social disruptions
- No utility involvement

4.8 Cost comparison

This section presents the analysis of the data that has been collected in this research.

Comparisons of methods used microtunneling and open cut – type

An analysis was performed to compare the cost of construction of open cut method and microtunnelling using recent projects of Colombo Municipality. The data was obtained from the submitted tenders and cost estimates of Project Management unit. Table gives the comparison.

Table 10 - Sri saddarma mawatha 375mm dia. pipe - open cut

Item No.	CESSM 3 Code No.	Description	Quantity	Unit	Rate (LKR)	Amount (LKR)	Remarks
Bill No. 4.1 - M26 (Siri Saddarma Mawatha)375mm dia. Pipe Open Cut							
CLASS A - GENERAL ITEMS							
		Field Test					
		Perform Field Test and Furnish					
4.1.1	A270	375mm dia. Clay pipe	10	m	165.61	165.61	
4.1.4	A271	Road traffic and safety management	Item	sum	20,107.96	20,107.96	
4.1.5	A273	Assess road maintenance	Item	sum	5,745.13	5,745.13	
4.1.6	A279	Tidiness of sites	Item	sum	2,872.57	2,872.57	
CLASS E - EARTH WORKS							
		Excavation and backfill with imported material and warning tape					
		Trench depth not exceeding 5-10m					
4.1.12	E425	Earth, Other than topsoil, rock or artificial hard material	50.63	m3	21,557.07	1,091,434.45	70% paid
4.1.13	E435	Rock required safety blasting.	2	m3	3,746.82	7,493.64	
4.1.14	E495	Artificial hard material.	2	m3	2,914.20	5,828.40	
CLASS I - PIPE WORK - PIPES							
		Supply, Laying and Fixing of Pipe					
		In trench depth 2-5m					
4.1.15	I139	375mm dia. Clay pipe	10	m	19,878.98	198,789.80	100% paid Due to preliminary testing report
		Road Reinstatement					
4.1.22	K732	Open trench for 375mm dia pipe	10	m	5,116.36	51,163.60	paid 75 %, wearing course not comp.
CLASS L - PIPE WORK - SUPPORTS AND PROTECTION ANCILLARIES TO LAYING AND EXCAVATION							
		Pipe bedding for open cut laying					
		Trench & Bedding Type - A (375mm dia pipe)					
4.1.28	L313	150mm thick ABC or compacted sand or selected material bad	10	m	393.42	3,934.20	
4.1.29	L513	Surrounding wet sand or selected as dug material	10	m	3,147.33	31,473.30	
		Total for 10.0m length				1,738,181.18	
		Total for 1.0m length				173,818.12	

Table 10 analyses a cost estimation to lay 375 mm diameter pipeline along Sri Saddarma mawatha, Colombo 09. It demonstrates direct costs incurred if the project had been carried out in open cut method.

Table 11 - Sri saddarma mawatha 375mmdia. pipe microtunneling

Item No.	CESSM 3 Code No.	Description	Quantity	Unit	Rate (LKR)	Amount (LKR)	Remarks
Bill No. 4.5 - M26 (Dematagoda Road) 375mm dia. Pipe Micro Tunnelling							
		CLASS A - GENERAL ITEMS					
		Field Test					
		Perform Field Test and Furnish					
4.5.3	A270	375mm dia. Clay pipe from Existing M102	10	m	165.61	1,656.10	
		Temporary Works					
4.5.4	A271	Road traffic and safety management	Item	sum	13,988.14	26,810.61	
4.5.5	A273	Assess road maintenance	Item	sum	3,996.61	7,660.17	
4.5.6	A279	Tidiness of sites	Item	sum	1,998.31	3,830.09	
		CLASS I - PIPE WORK - PIPES					
		Supply, Laying and Fixing Pipes					
		In trench depth 5-10m					
4.5.9	I139	375mm dia. Clay pipe from Existing M102	10	m	19,878.98	198,789.80	
		CLASS L - PIPE WORK - SUPPORTS AND PROTECTION ANCILLARIES TO LAYING AND EXCAVATION					
		Trenchless Excavation					
		Micro tunneling					
		In depth 5-10m					
4.5.16	L292	375mm dia. Clay pipe from Existing M102	10	m	238,680.32	2,386,803.20	
		Total for 10.0m length				2,625,549.97	
		Total for 1.0m length				262,555.00	

Table 11 analyses cost estimation of the same project if it had been conducted by implementing Microtunneling technology.

Table 12 - Maligawatte railway quarters road 600mm dia. pipe open cut

Item No.	CESSM 3 Code No.	Description	Quantity	Unit	Rate (LKR)	Amount (LKR)	Remarks
Bill No. 4.2 - M26 (Railway Quarters Road) 600mm dia. Pipe Open cut							
CLASS A - GENERAL ITEMS							
		Field Test					
		Perform Field Test and Furnish					
4.2.1	A270	600mm dia. Clay pipe	287	m	327.97	94,127.39	
		Temporary Works					
4.2.2	A271	Road traffic and safety management	Item	sum	167,274.88	167,274.88	
4.2.3	A273	Assess road maintenance	Item	sum	47,792.82	47,792.82	
4.2.4	A279	Tidiness of sites	Item	sum	23,896.41	23,896.41	
CLASS E - EARTH WORKS							
		Excavation trench depth not exceeding 5-10m					
4.2.6	E425	Earth, Other than topsoil, rock or artificial hard material	2583	m3	22,565.02	58,285,446.66	
4.2.7	E435	Rock required safety blasting	50	m3	3,746.82	187,341.00	
4.2.8	E495	Artificial hard material	50	m3	2,914.20	145,710.00	
CLASS I - PIPE WORK - PIPES							
		Supply, Laying and Fixing of Pipe					
		In trench depth 5-10m					
4.2.9	I129	600mm dia. Clay pipe	287	m	39,461.55	11,325,464.85	
		Road Reinstatement					
4.2.12	K732	Open trench for 600mm dia pipe	287	m	6,297.06	1,807,256.22	
CLASS L - PIPE WORK - SUPPORTS AND PROTECTION ANCILLARIES TO LAYING AND EXCAVATION							
		Trench & Bedding Type - E (600mm dia pipe)					
4.2.16	L322	75mm thick stabilized beds	287	m	240.42	69,000.54	
4.2.17	L542	G20 concrete for pipe encasement	287	m	19,583.40	5,620,435.80	
4.2.18	L522	Surrounding graded granular fill	287	m	1,223.96	351,276.52	
Total for 287.0m length						78,125,023.09	
Total for 1.0m length						272,212.62	

Table 12 analyses a cost estimation to lay 600 mm diameter pipeline along Maligawatte Railway Quarters Road, Colombo 09. It demonstrates direct costs incurred if the project had been carried out in open cut method.

4.8.2. Summary of comparison

M26 Sewer line along Siri Saddamma Mawatha - 375mm dia. Pipe

- Open cut – total cost for 1m length Rs. 173,818.12
- Microtunneling – total cost for 1m length Rs. 262,555.00

M26 Sewer line along Railway Quarters Road - 600mm dia. Pipe

- Open cut – total cost for 1m length Rs. 272,212.62
- Microtunneling – total cost for 1m length Rs. 315,444.33

However, it could be also seen that this the cost difference between the two methods is generally less than LKR 100,00.00 to lay a pipe of 1 metre length.

Analysis of qualitative data gathered in interviews conducted with five experts involved in these construction projects revealed the following benefits;

- Reduced disruption of the community.
- Reduced liability for personal injury and property damage.
- Increased service life and asset value for the utility owner.
- Increased worker safety.
- Reduced restoration costs.
- Precise installation.
- Faster rate of progress than conventional tunneling
- Minimum interruption to traffic.
- High quality rehabilitated sewer
- Time saving, minimum earth disturbances
- Fifty years of design life, easy ground water handling

It is a complicated and complex task to calculate numerical values of costs incurred in each of these themes. For an example, costs incurred in interruption to traffic is difficult to quantify and calculate. However, it was also evidenced through these interviews that the indirect costs incurred in the aforementioned key themes when compared with the difference between the costs of open-cut and Microtunneling methods were significantly higher. Therefore, the total costs (direct costs, indirect costs and social costs) incurred in open-cut method is significantly higher when compared with the total cost of Microtunneling method.

4.9 Parameters for analysis for applications in Sri Lanka

Traffic related data can be obtained from Traffic Division CMC. Data related to usual traffic condition is available. However data related to delays due to construction obstacles is not available. Hence I could not calculate the impact of traffic delays. Project feasibility reports also do not have analysis of traffic delays and other social parameters.

Therefore, these analysis and data need to be collected in future studies for viability of microtunneling.

Vehicle turning movements at the traffic light junctions

Information based on the survey conducted to calculate vehicle turning movements at the following traffic light junctions are illustrated in Figures 47-55.

- R A De Mel Mawatha / Bauddhaloka Mawatha Junction
- Vajira Road / Sri Sambuddhathva Jayanthi Mawatha Junction
- Sri Sambuddhathva Jayanthi Mawatha / Thimbirigasyaya Road Junction
- Isipathana Mawatha / Fife Road Junction
- Galle Road / Bauddhaloka Mawatha Junction
- Kanaththa Junction
- Borella Junction
- Rajagiriya Junction
- Panchikawatta Junction

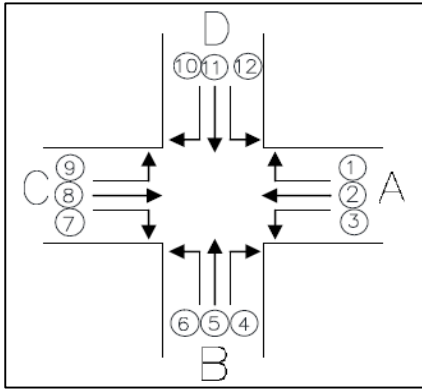


Figure 47 - R A D Mel mawatha Bulls road junction

- A. R A De Mel Mw (From Kollupitiya)
- B. Bauddhaloka Mw (From Thunmulla Jn)
- C. R A De Mel Mw (From Wellawatta)
- D. Bauddhaloka Mw (From Bambalapitiya Jn)

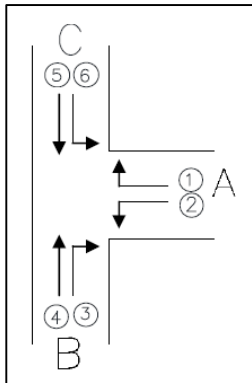


Figure 48 - Vajira road / R A D Mel mawatha junction

- A. Vajira Rd (From R A De Mel Mw Jn)
- B. Sri Sambuddhathva Jayanthi Mw (From Thummulla Jn)
- C. Sri Sambuddhathva Jayanthi Mw (From Thimbirigasyaya Jn)

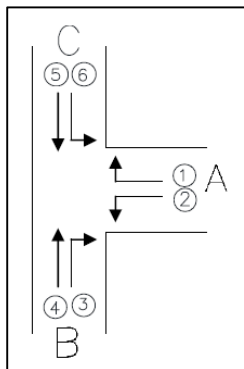


Figure 49 - Thimbirigasyaya road / Sri Sambuddhathva Jayanthi mawatha junction

- A. Thimbirigasyaya Rd (From Thimbirigasyaya Jn)
- B. Sri Sambuddhathva Jayanthi Mw (From Dickmans Rd)
- C. Sri Sambuddhathva Jayanthi Mw (From Thummulla Jn)

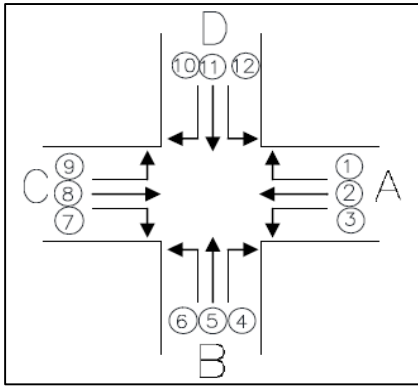


Figure 50- Sri Sambuddhathva Jayanthi mawatha / Isipathana mawatha junction

- A. Sri Sambuddhathva Jayanthi Mw (From Kirulapone Jn)
- B. Dr Lester James Peries Mw (From R A De Mel Mw Jn)
- C. Sri Sambuddhathva Jayanthi Mw (From Thimbirigasyaya Jn)
- D. Isipathana Mw (From Fife Rd Jn)

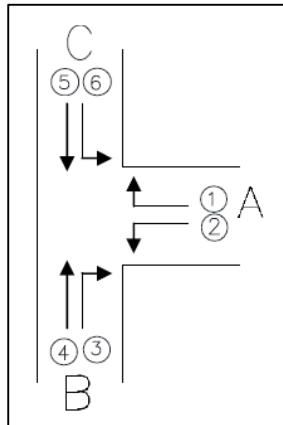


Figure 51 - Bambalapitiya junction

- A. Bauddhaloka Mw (From Duplication Road Jn)
- B. Galle Rd (From Wellawatta Jn)
- C. Galle Rd (From Kollupitiya Jn)

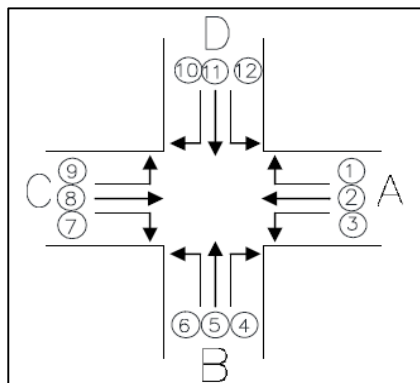


Figure 52 - Ayurveda junction

- A. Sri Jayawardanapura Mawatha (From Rajagiriya Jn)
- B. Sri Jayawardanapura Mawatha (From Kanatta Jn)
- C. Dr.N. M. Perera Mawatha (From Borella Jn)
- D. Borella - Rajagiriya Road (From Rajagiriya Jn)

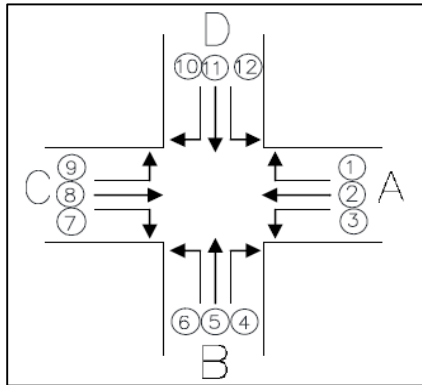


Figure 53 - Devi Balika junction

- A. Sri Jayawardanapura Mawatha
(From Rajagiriya Jn)
- B. Bauddaloka Mawatha (From
Kanatta Jn)
- C. Dudly Senanayaka Mawatha (From
Elvitigala MW Jn)
- D. Bauddaloka Mawatha (From Dr. N.
M. Perera Mw Jn)

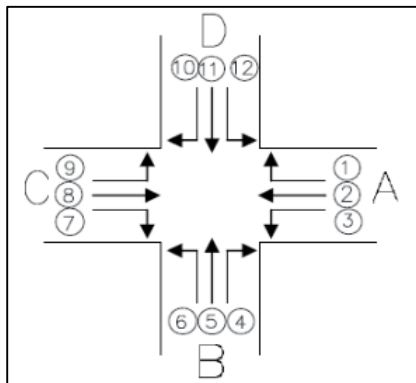
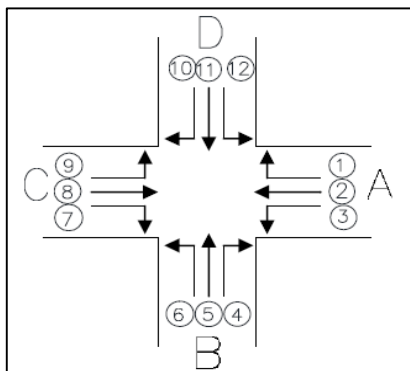


Figure 54 - Kanaththa junction

- D S Senanayake Mw (From
Kanatta Jn)
- A. Dudly Senanayake Mw (From
Kinsey Rd Jn)
- B. D S Senanayake Mw (From
Borella Jn)
- C. Dudly Senanayake Mw (From
Bauddhaloka Mw Jn)



**Figure 55 -
Panchikawaththa junction**

- A. Sri Sumanatissa Mw.
(From Panchikawatta)
- B. M J M Lafeer Mw.
- C. Sri Sumanatissa Mw.
(From Armour Street Jn)
- D. M Vincent Perera Mw.

Appendix 1 summarises travelling movements at each hour of the day at each of these junctions to present number of vehicles travelled through the traffic light junctions. These data clearly demonstrate the magnitude of traffic occurred at each of these junctions. During the expert interviews, interviewees pointed out that to pass a traffic flow of this magnitude through an open-cut construction excavation at these junctions would result in extremely huge traffic. This kind of extremely huge traffic would result in high indirect costs in areas including but not limited to, travelling time, mental and physical fatigue of travellers. These indirect costs are unmeasurable and unquantifiable, and therefore its impact is unpredictable when calculating the total cost.

Analysing all of these factors, it could be concluded that even though the direct costs incurred in open-cut constructions are lower than Microtunneling constructions, when all the indirect costs are considered the total cost of open-cut construction method is significantly higher, unmeasurable and the cost-impact is very long-term compared with the open-cut construction method.

5 CONCLUSIONS AND RECOMMENDATIONS

5.1 Technology

The following technologies have been used in Sri Lanka and recorded success.

- Pipe jacking method used by MTBM – Mainly used for gravity storm water project, sewer projects & for when using rigid pipes
- Other methods -Horizontal Directional Drilling mainly used for force main part in sewer projects
- Pipe jacking method and machine used is horizontal jacking and Boring Unit.

Although Engineers are responsible for the development of new construction methods and technology, there is also resistance among the Engineering community to accept new technologies. The resistance to adopt new technology could be due to many reasons such as, risk involved, un-familiarity with the new technology and the misconception that new technology is always expensive than the traditional methods. In respect of the new technology of which I intend to research does not have much information of the cost involved?

5.2 Cost parameters

Even though the direct costs incurred in open-cut constructions are lower than Microtunneling constructions, when all the indirect & social costs are considered, the total cost of open-cut construction method is significantly higher, unmeasurable and the cost-impact of the Microtunneling method is very long-term compared with the open-cut construction method.

5.3 Framework for critical analysis of Microtunneling in Sri Lanka.

I believe that further studies shall be necessary in order to evaluate the advantages and disadvantages of Microtunneling technology. Therefore, I believe that a cost comparison would be extremely beneficial comparing trenchless technology and open cut method could be beneficial for future projects. I also wish to state that some of the costs that are available are not comparable with the cost as in developing countries, cost of technology while be expensive, cost of labour is comparatively less expensive.

Furthermore, people are demanding for better quality of life, efficient and effective service facilities and time has come to replace and expand the underground utility facilities, specially provided by the water sector. In order to expand the water and waste water facilities, open cut excavation methods have become obsolete and therefore, more emphasis has to be given to Microtunneling techniques which would save existing aboveground facilities such as, sidewalks, pavements, landscaping etc.

Furthermore, there is an additional social and environmental factors in the open trench methods. Trenchless technologies minimises surface excavation and this method could be used to repair, upgrade, replace and renovate underground infrastructure systems. This technology is very commonly used in developing countries and it is a viable option for Sri Lankan cities as well.

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Appendix 1

Table 14 - hourly distribution of traffic movements by all approach links at junction No. 5

Technical Assistance for ATMS in Colombo Metropolitan Region																				
Hourly Distribution of Traffic Movements By All Approach Links at Junction No 5																				
Time		Link "A"						Link "B"						Link "C"						
		Out Bound		In Bound		Total	Out Bound		In Bound		Total	Out Bound		In Bound		Total				
		3	6	Total	1	2	Total	2	5	Total	3	4	Total	1	4	Total	5	6	Total	
06:00	-	270	146	416	289	190	479	190	714	904	270	1204	1474	289	1204	1493	714	146	860	
07:00	-	481	303	784	595	400	995	400	1199	1599	481	1892	2373	595	1892	2487	1199	303	1502	
08:00	-	415	275	690	418	406	824	406	754	1160	415	2252	2667	418	2252	2670	754	275	1029	
09:00	-	375	290	665	504	478	982	478	702	1180	375	1544	1919	504	1544	2048	702	290	992	
10:00	-	327	280	607	383	349	732	349	1260	1609	327	1433	1760	383	1433	1816	1260	280	1540	
11:00	-	376	375	751	548	493	1041	493	1279	1772	376	2885	3261	548	2885	3433	1279	375	1654	
12:00	-	377	375	752	527	417	944	417	1413	1830	377	1816	2193	527	1816	2343	1413	375	1788	
13:00	-	411	389	800	528	411	939	411	1721	2132	411	2359	2770	528	2359	2887	1721	389	2110	
14:00	-	359	341	700	494	318	812	318	1473	1791	359	1438	1797	494	1438	1932	1473	341	1814	
15:00	-	290	242	532	333	299	632	299	1058	1357	290	1217	1507	333	1217	1550	1058	242	1300	
16:00	-	414	294	708	421	424	845	424	1873	2297	414	1600	2014	421	1600	2021	1873	294	2167	
17:00	-	411	379	790	454	513	967	513	2345	2858	411	1375	1786	454	1375	1829	2345	379	2724	
18:00	-	323	322	645	342	462	804	462	1959	2421	323	1135	1458	342	1135	1477	1959	322	2281	
19:00	-	357	284	641	273	328	601	328	1528	1856	357	663	1020	273	663	936	1528	284	1812	
20:00	-	235	226	461	178	219	397	219	1111	1330	235	686	921	178	686	864	1111	226	1337	
21:00	-	145	74	219	97	52	149	52	499	551	145	220	365	97	220	317	499	74	573	
Totals		5566	4595	10161	6384	5759	12143	5759	20888	26647	5566	23719	29285	6384	23719	30103	20888	4595	25483	
		Total In Bound			66911															
		Total Out Bound			66911															

Table 16 - hourly distribution of traffic movements by all approach links at junction No. 6

Time		Link "A"												Link "B"												Link "C"												Link "D"											
		Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound																				
		4	8	12	Total	1	2	3	Total	3	7	11	Total	4	5	6	Total	2	6	10	Total	7	8	9	Total	1	5	9	Total	10	11	12	Total																
06:00	-	70	579	54	703	0	1195	53	1248	53	108	167	328	70	193	111	374	115	115	1421	108	108	579	45	732	0	193	45	238	115	167	54	336																
07:00	-	142	926	115	1183	0	1707	47	1754	47	212	353	612	142	284	255	681	256	256	2218	212	212	926	175	1313	0	284	175	459	256	353	115	724																
08:00	-	166	916	118	1200	0	1588	55	1643	55	230	329	614	166	233	274	673	274	177	2039	230	230	916	95	1241	0	233	95	328	177	329	118	624																
09:00	-	168	1005	110	1283	0	1707	85	1792	85	273	238	596	168	199	213	580	213	153	2073	273	273	1005	93	1371	0	199	93	292	153	238	110	501																
10:00	-	190	1146	126	1462	0	1162	84	1246	84	190	216	490	190	210	195	595	195	141	1498	190	190	1146	97	1433	0	210	97	307	141	216	126	483																
11:00	-	225	929	121	1275	0	1670	146	1816	146	217	217	580	225	168	256	649	256	165	2091	217	217	929	120	1266	0	168	120	288	165	217	121	503																
12:00	-	239	1255	155	1649	0	1454	150	1604	150	212	256	618	239	274	279	792	279	168	1901	212	212	1255	117	1584	0	274	117	391	168	256	155	579																
13:00	-	271	1222	144	1637	0	1324	121	1445	121	256	317	694	271	370	309	950	309	228	1861	256	256	1222	134	1612	0	370	134	504	228	317	144	689																
14:00	-	201	1021	106	1328	0	1244	127	1371	127	175	254	556	201	248	244	693	244	157	1645	175	175	1021	111	1307	0	248	111	359	157	254	106	517																
15:00	-	164	981	112	1257	0	986	73	1059	73	167	192	432	164	184	170	518	170	127	1283	167	167	981	86	1234	0	184	86	270	127	192	112	431																
16:00	-	281	1772	176	2229	0	1323	103	1426	103	223	271	597	281	304	208	793	208	165	1696	223	223	1772	153	2148	0	304	153	457	165	271	176	612																
17:00	-	308	1975	233	2516	0	1301	81	1382	81	324	287	692	308	388	212	908	212	188	1701	324	324	1975	137	2436	0	388	137	525	188	287	233	708																
18:00	-	283	2064	239	2586	0	1031	91	1122	91	208	215	514	283	211	166	660	166	150	1347	208	208	2064	91	2363	0	211	91	302	150	215	239	604																
19:00	-	212	1556	150	1918	0	785	64	849	64	196	137	397	212	121	149	482	149	95	1029	196	196	1556	66	1818	0	121	66	187	95	137	150	382																
20:00	-	149	917	101	1167	0	556	48	604	48	120	101	269	149	107	120	376	120	73	749	120	120	917	57	1094	0	107	57	164	73	101	101	275																
21:00	-	66	807	56	929	0	129	15	144	15	46	35	96	66	32	61	159	61	35	225	46	46	807	26	879	0	32	26	58	35	56	126	126																
Totals		3135	19071	2116	24322	0	19162	1343	20505	1343	3157	3585	8085	3135	3526	9883	3222	2399	24777	3157	19071	1603	23831	0	3526	1603	5129	2393	3585	2116	8094																		
		Total In Bound												62313																																			
		Total Out Bound												62313																																			

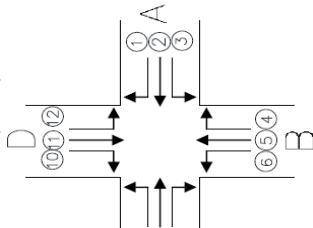
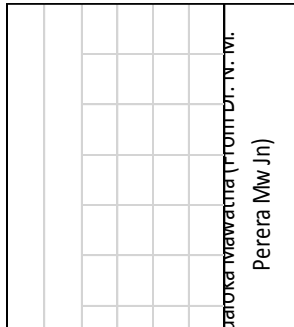


Table 17 - hourly distribution of traffic movements by all approach links at junction No. 56

Hourly Distribution of Traffic Movements By All Approach Links at Junction No 56																		
Buddhaloka Mw (From Duplication Road Jn)																		
Galle Rd (From Wellawatta Jn)																		
Galle Rd (From Kollupitiya Jn)																		
Link "A"			Link "B"			Link "C"			Link "A"			Link "B"			Link "C"			
Time	Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound		
	3	6	Total	1	2	Total	2	5	Total	3	4	Total	1	4	Total	5	6	Total
06:00	0	0	0	468	0	468	0	0	0	0	0	0	468	0	0	0	0	0
07:00	0	0	0	1146	0	1146	0	0	0	0	0	0	1146	0	0	0	0	0
08:00	0	0	0	1047	0	1047	0	0	0	0	0	0	1047	0	0	0	0	0
09:00	0	0	0	1060	0	1060	0	0	0	0	0	0	1060	0	0	0	0	0
10:00	0	0	0	826	0	826	0	0	0	0	0	0	826	0	0	0	0	0
11:00	0	0	0	1088	0	1088	0	0	0	0	0	0	1088	0	0	0	0	0
12:00	0	0	0	1199	0	1199	0	0	0	0	0	0	1199	0	0	0	0	0
13:00	0	0	0	1168	0	1168	0	0	0	0	0	0	1168	0	0	0	0	0
14:00	0	0	0	1035	0	1035	0	0	0	0	0	0	1035	0	0	0	0	0
15:00	0	0	0	727	0	727	0	0	0	0	0	0	727	0	0	0	0	0
16:00	0	0	0	1013	0	1013	0	0	0	0	0	0	1013	0	0	0	0	0
17:00	0	0	0	1045	0	1045	0	0	0	0	0	0	1045	0	0	0	0	0
18:00	0	0	0	788	0	788	0	0	0	0	0	0	788	0	0	0	0	0
19:00	0	0	0	686	0	686	0	0	0	0	0	0	686	0	0	0	0	0
20:00	0	0	0	513	0	513	0	0	0	0	0	0	513	0	0	0	0	0
21:00	0	0	0	311	0	311	0	0	0	0	0	0	311	0	0	0	0	0
Totals	0	0	0	14120	0	14120	0	0	0	0	0	0	22491	14120	22491	0	0	36611
Total In Bound															36611			
Total Out Bound															36611			

Table 19 - hourly distribution of traffic movements by all approach links at junction No. 27



Technical Assistance for ATMS in Colombo Metropolitan Region																																
Hourly Distribution of Traffic Movements By All Approach Links at Intersection No 27																																
Time	Link "A"				Link "B"				Link "C"				Link "D"				Total															
	Out Bound	In Bound	Total		Out Bound	In Bound	Total		Out Bound	In Bound	Total		Out Bound	In Bound	Total																	
06:00	209	525	0	734	0	640	1296	1936	1296	47	0	1343	209	58	3	270	640	3	82	725	47	525	22	594	0	58	22	80	82	0	0	82
07:00	461	939	0	1400	1	821	1606	2428	1606	159	0	1765	461	136	12	609	821	12	66	899	159	939	54	1152	1	136	54	191	66	0	0	66
08:00	569	803	0	1372	0	1259	1779	3038	1779	117	0	1896	569	137	9	715	1259	9	95	1363	117	803	39	959	0	137	39	176	95	0	0	95
09:00	499	667	0	1166	0	1783	2064	3847	2064	73	0	2137	499	98	4	601	1783	4	81	1868	73	667	36	776	0	98	36	134	81	0	0	81
10:00	678	929	0	1607	0	949	1010	1959	1010	75	0	1085	678	140	16	834	949	16	66	1031	75	929	59	1063	0	140	59	199	66	0	0	66
11:00	466	747	0	1213	1	1208	1114	2323	1114	104	0	1218	466	110	21	597	1208	21	63	1292	104	747	89	940	1	110	89	200	63	0	0	63
12:00	870	1133	0	2003	0	1436	1400	2836	1400	114	0	1514	870	142	30	1042	1436	30	85	1551	114	1133	99	1346	0	142	99	241	85	0	0	85
13:00	1240	1073	0	2313	0	1311	1658	2969	1658	116	0	1774	1240	189	16	1445	1311	16	78	1405	116	1073	141	1330	0	189	141	330	78	0	0	78
14:00	1132	707	0	1839	0	1174	1337	2511	1337	99	0	1436	1132	262	10	1404	1174	10	76	1260	99	707	143	949	0	262	143	405	76	0	0	76
15:00	667	919	0	1586	0	931	975	1906	975	73	0	1048	667	135	15	817	931	15	65	1011	73	919	57	1049	0	135	57	192	65	0	0	65
16:00	871	1305	0	2176	0	1197	970	2167	970	78	0	1048	871	173	8	1052	1197	8	87	1292	78	1305	28	1411	0	173	28	201	87	0	0	87
17:00	1148	1820	0	2968	0	1114	961	2075	961	67	0	1028	1148	232	36	1416	1114	36	68	1218	67	1820	47	1934	0	232	47	279	68	0	0	68
18:00	1156	1790	0	2946	0	754	0	754	0	0	0	1156	208	3	1367	754	3	34	791	0	1790	41	1831	0	208	41	249	34	0	0	34	
19:00	820	1373	0	2193	0	566	0	566	0	0	0	820	144	3	967	566	3	24	593	0	1373	42	1415	0	144	42	186	24	0	0	24	
20:00	405	671	0	1076	0	485	501	986	501	35	0	536	405	65	8	478	485	8	31	524	35	671	26	732	0	65	26	91	31	0	0	31
21:00	221	415	0	636	0	491	294	785	294	40	0	334	221	40	4	265	491	4	33	528	40	415	21	476	0	40	21	61	33	0	0	33
Totals	11412	15816	0	27228	2	16119	16965	33086	16965	1197	0	18162	11412	2269	198	13879	16119	198	1034	17351	1197	15816	944	17957	2	2269	944	3215	1034	0	0	1034
Total In Bound																									65956							
Total Out Bound																									65956							

Table 20 - hourly distribution of traffic movements by all approach links at junction No.. 46

Time		Technical Assistance for ATMS in Colombo Metropolitan Region																																													
		Hourly Distribution of Traffic Movements By All Approach Links at Intersection No 46																																													
		D S Senanayake Mw (From Kanatta.Jn)												Dudly Senanayake Mw (From Kinsey Rd Jn)												D S Senanayake Mw (From Borella.Jn)												Dudly Senanayake Mw (From Baudhaloka Mw Jn)									
Link "A"												Link "B"												Link "C"												Link "D"											
Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound														
4	8	12	Total	1	2	3	Total	3	7	11	Total	4	5	6	Total	2	6	10	Total	7	8	9	Total	1	5	9	Total	10	11	12	Total																
06:00	0	630	29	659	0	581	10	591	0	641	651	0	969	22	991	581	22	181	784	0	630	36	666	0	969	36	1005	181	641	29	851																
07:00	0	1309	30	1339	0	1016	16	1032	16	1310	1326	0	1346	22	1368	1016	22	262	1300	0	1309	41	1350	0	1346	41	1387	262	1310	30	1602																
08:00	0	1341	18	1359	0	1272	13	1285	13	0	1592	1605	0	861	37	898	1272	37	269	1578	0	1341	60	1401	0	861	60	921	269	1592	18	1879															
09:00	0	1374	18	1392	0	1200	22	1222	22	0	1559	1581	0	943	27	970	1200	27	326	1553	0	1374	80	1454	0	943	80	1023	326	1559	18	1903															
10:00	0	1072	104	1176	0	862	17	879	17	0	822	839	0	1035	39	1074	862	39	191	1092	0	1072	83	1155	0	1035	83	1118	191	822	104	1117															
11:00	0	1216	88	1304	0	977	21	998	21	0	1046	1067	0	1001	62	1063	977	62	237	1276	0	1216	105	1321	0	1001	105	1106	237	1046	88	1371															
12:00	0	1230	95	1325	0	1128	20	1148	20	0	1061	1081	0	1154	65	1219	1128	65	264	1457	0	1230	110	1340	0	1154	110	1264	264	1061	95	1420															
13:00	0	1238	86	1324	0	949	13	962	13	0	939	952	0	1318	36	1354	949	36	276	1261	0	1238	84	1322	0	1318	84	1402	276	939	86	1301															
14:00	0	1330	80	1410	0	968	13	981	13	0	918	931	0	1438	54	1492	968	54	193	1215	0	1330	82	1412	0	1438	82	1520	193	918	80	1191															
15:00	0	1034	101	1135	0	830	18	848	18	0	824	842	0	1037	37	1074	830	37	191	1058	0	1034	82	1116	0	1037	82	1119	191	824	101	1116															
16:00	0	1082	79	1161	0	841	21	862	21	0	846	867	0	1204	33	1237	841	33	191	1065	0	1082	76	1158	0	1204	76	1280	191	846	79	1116															
17:00	0	1035	83	1118	0	1007	22	1029	22	0	846	868	0	1586	39	1625	1007	39	182	1228	0	1035	99	1134	0	1586	99	1685	182	846	83	1111															
18:00	0	1409	337	1746	0	981	18	999	18	0	658	676	0	1562	36	1598	981	36	176	1193	0	1409	170	1579	0	1562	170	1732	176	658	337	1171															
19:00	0	1189	336	1525	0	812	10	822	10	0	657	667	0	1122	10	1132	812	10	125	947	0	1189	91	1280	0	1122	91	1213	125	657	336	1118															
20:00	0	705	101	806	0	503	11	514	11	0	384	395	0	718	22	740	503	22	83	608	0	705	60	765	0	718	60	778	83	384	101	568															
21:00	0	413	75	488	0	337	7	344	7	0	157	164	0	430	17	447	337	17	49	403	0	413	52	465	0	430	52	482	49	157	75	281															
Totals	0	17607	1660	19267	0	14264	252	14516	252	0	14260	14512	0	17724	558	18282	14264	558	3196	18018	0	17607	1311	18918	0	17724	1311	19035	3196	143260	1660	19116															
Total In Bound																								70832																							
Total Out Bound																								70832																							

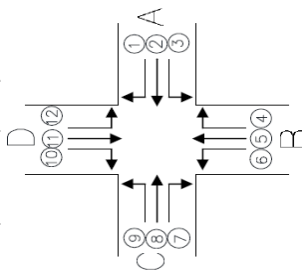


Table 22 - hourly distribution of traffic movements by all approach links at junction No. 58

		Technical Assistance for ATMS in Colombo Metropolitan Region																																	
		Hourly Distribution of Traffic Movements By All Approach Links at Intersection No 58																																	
		Link "A"						Link "B"						Link "C"						Link "D"															
Time		Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound			Out Bound			In Bound												
		4	8	12	Total	1	2	3	Total	3	7	11	Total	4	5	6	Total	2	6	10	Total	7	8	9	Total	1	5	9	Total	10	11	12	Total		
06:00	-	0	0	0	0	46	466	82	594	82	0	0	82	0	268	198	466	466	198	0	664	0	0	0	0	0	0	0	46	268	0	314	0	0	0
07:00	-	0	0	0	0	97	996	413	1506	413	0	0	413	0	772	247	1019	996	247	0	1243	0	0	0	0	0	0	97	772	0	869	0	0	0	
08:00	-	0	0	0	0	98	1113	656	1867	656	0	0	656	0	803	282	1085	1113	282	0	1395	0	0	0	0	0	98	803	0	901	0	0	0		
09:00	-	0	0	0	0	104	1252	497	1853	497	0	0	497	0	496	246	742	1252	246	0	1498	0	0	0	0	0	104	496	0	600	0	0	0		
10:00	-	0	0	0	0	74	1002	494	1570	494	0	0	494	0	588	243	831	1002	243	0	1245	0	0	0	0	0	74	588	0	662	0	0	0		
11:00	-	0	0	0	0	115	1028	233	1376	233	0	0	233	0	574	222	796	1028	222	0	1250	0	0	0	0	0	115	574	0	689	0	0	0		
12:00	-	0	0	0	0	122	1098	823	2043	823	0	0	823	0	900	314	1214	1098	314	0	1412	0	0	0	0	0	122	900	0	1022	0	0	0		
13:00	-	0	0	0	0	75	1092	807	1974	807	0	0	807	0	867	276	1143	1092	276	0	1368	0	0	0	0	0	75	867	0	942	0	0	0		
14:00	-	0	0	0	0	94	812	629	1535	629	0	0	629	0	779	279	1058	812	279	0	1091	0	0	0	0	0	94	779	0	873	0	0	0		
15:00	-	0	0	0	0	63	836	421	1320	421	0	0	421	0	508	211	719	836	211	0	1047	0	0	0	0	0	63	508	0	571	0	0	0		
16:00	-	0	0	0	0	74	1334	760	2168	760	0	0	760	0	707	281	988	1334	281	0	1615	0	0	0	0	0	74	707	0	781	0	0	0		
17:00	-	0	0	0	0	80	1370	903	2353	903	0	0	903	0	680	333	1013	1370	333	0	1703	0	0	0	0	0	80	680	0	760	0	0	0		
18:00	-	0	0	0	0	50	1474	671	2195	671	0	0	671	0	566	335	901	1474	335	0	1809	0	0	0	0	0	50	566	0	616	0	0	0		
19:00	-	0	0	0	0	27	1130	372	1529	372	0	0	372	0	732	228	960	1130	228	0	1358	0	0	0	0	0	27	732	0	759	0	0	0		
20:00	-	0	0	0	0	48	690	218	956	218	0	0	218	0	321	185	506	690	185	0	875	0	0	0	0	0	48	321	0	369	0	0	0		
21:00	-	0	0	0	0	21	616	145	782	145	0	0	145	0	207	160	367	616	160	0	776	0	0	0	0	21	207	0	228	0	0	0			
Totals		0	0	0	0	1188	16309	8124	25621	8124	0	8124	0	8124	0	9768	4040	13808	16309	4040	20349	0	0	0	0	1188	9768	0	10956	0	0	0			
		Total In Bound						39429																											
		Total Out Bound						39429																											

