

**EFFECTIVENESS OF SUBSURFACE DRAINAGE AND
VEGETATION IN ENHANCING THE SLOPE STABILITY:
A COMPREHENSIVE CASE STUDY ON
BADLUSIRIGAMA LANDSLIDE**

Lilanka Udayana Matarambha Kankanamge

(168963X)

Thesis submitted in partial fulfilment of the requirements for the degree Master of
Engineering in Foundation Engineering and Earth Retaining Systems

Department of Civil Engineering

University of Moratuwa

Sri Lanka

July 2020

Statement of Authentication

I declare that this is my own work and this thesis does not incorporate without acknowledgement any material previously submitted for a Degree or Diploma in any other University or institute of higher learning and to the best of my knowledge and belief it does not contain any material previously published or written by another person except where the acknowledgement is made in the text.

Also, I hereby grant to University of Moratuwa the non-exclusive right to reproduce and distribute my thesis/dissertation, in whole or in part in print, electronic or other medium. I retain the right to use this content in whole or part in future works (such as articles or books).

Name of the candidate: Lilanka Kankanamge

Signature of the candidate:

Date:

The above candidate has carried out research for the Master thesis under my supervision.

Signature of the supervisor:

Date:

Professor S.A.S Kulathilaka,
BSc. Eng. Hons (Moratuwa), PhD (Monash), CEng, MIE (SL)
Senior Professor,
Department of Civil Engineering,
University of Moratuwa,
Sri Lanka

Acknowledgement

This accomplishment would not have been a reality without the tremendous support I received from my supervisor, Senior Professor S.A.S. Kulathilaka. I am deeply grateful to him for setting a strong platform for me to pursue my career in geotechnical engineering and for building- up confidence in me to engage in innovative research activities. He not only encouraged but also facilitated the pathway for me to connect with other researchers from different parts of the world, to gain inspiration and to work on new research ideas. Professor Kulathilaka has been a true mentor and an exceptional role model to me throughout my research candidature.

Next, I would like to acknowledge Prof. Apiniti Jotisankasa of Kasetsart University, Thailand, for his suggestions, insightful comments and his generosity in sharing knowledge. My sincere thanks also extend to Dr. U.P. Nawagamuwa, Dr. Nalin de Silva, all the lecturers of the M.Eng programme and staff of the geotechnical engineering division of University of Moratuwa, for their assistance during my candidature.

It is my obligation to acknowledge the guidance and the support given by Eng. (Dr.) Asiri Karunawardena, the director general of NBRO and Mr. K.N. Bandara, the director of the Geotechnical Engineering Division (GED) of NBRO. Also, the support from all the staff of GED, especially from Eng (Ms). Nanthini Vasanthan, Mr. Chandima, Mr. Abeysinghe, Mr. Prasad and Ms. Sewwandi is thankfully appreciated.

The continuous assistance, kindness and the understanding from my mother, father and brother has helped me to come a long way and it meant a lot to me during my research career as well. I would also like to thank my friends and colleagues who never failed to cheer me up when it is most needed.

Last but not least, I would like to express my gratitude to Eng. (Dr.) Manasi Wijerathna for being with me in every ups and downs and supporting me on the way to this great achievement that I am proud of.

Abstract

Slope instability, triggered by excessive rainfall, is one of the common geo –hazards that geotechnical engineers are challenged with in tropical countries such as Sri Lanka. Typically, these slope failures are initiated in colluvial layers derived from former landslides or planes of low shear strength in differently weathered zones in the thick soil overburden. Improvement of surface and subsurface drainage has proven to be effective in improving the slope stability by lowering the ground water table as well as preventing near surface perched water table conditions. Badulusirigama Landslide in central highlands of Sri Lanka is an example for a slow moving long rotational slip that activates after heavy rainfall events.

The landslide was rectified with over 45 m long individual sub-horizontal drains that are arranged into a network of radial drainage groups at different elevations along the long sliding mass. This site is also well equipped with monitoring instruments and thus provides a great case history to further our understanding on contribution of surface and subsurface drains in mitigating landslides. In this study, the effectiveness of the introduced subsurface drainage measures in enhancing the stability of the Badulusirigama Landslide was investigated using 2D and 3D numerical models. The numerical models were then used to predict the behaviour of the landslide during different anticipated rainfall events.

The results show that the subsurface drains system enhances the initial near failure condition of the site to a stable slope with a factor of safety of over 1.25 within one month. After initial drop down of the ground water table, the sub-horizontal drains still remain effective by rapidly draining out any infiltration. The analysis also shows that the width of the influence zone of radial horizontal drains should be carefully selected when simplifying the problem into 2D plane strain models because the influence can be very much localised in a low permeable medium. Possibility of introducing surface vegetation as a hybrid measure along with subsurface drainage was also investigated. A factor of safety improvement of 38% and 16.3% was achieved after the simulation of construction of the drains in 2D plane strain and 3D finite element analyses separately. Also, it was found that, vegetation could result in increasing the hydraulic conductivity of the root zone, leading to development of perched water table conditions.

Table of Contents

Statement of Authentication	i
Acknowledgement	ii
Abstract.....	iii
List of Figures.....	x
List of Tables	xvii
1 INTRODUCTION.....	1
1.1 Background	1
1.2 Scope of the research	3
1.3 Thesis objectives	3
1.4 Research methodology	3
1.5 Thesis outline	5
2 REVIEW OF STUDIES ON RAIN INDUCED SLOPE INSTABILITY AND STABILIZATION METHODS	7
2.1 Introduction.....	7
2.2 Stability of a slope	7
2.3 Methods of slope stability assessment.....	9
2.3.1 Limit Equilibrium Method (LEM) of Slope Stability Analysis	10
2.3.2 Finite Element Method (FEM) to Assess the Stability of Slopes	16
2.4 Slope Stabilization Techniques	20
2.4.1 Surface Drainage Measures	20
2.4.2 Sub Surface Drainage in Slope stability	21

2.4.3	Landslide Mitigation using Sub Surface Drainage Improvements in Sri Lanka	30
2.4.4	Modelling the Effects of Sub Surface Drains in a Plain Strain Formulation	37
2.5	Effect of Vegetation on Stability of Slopes	40
2.5.1	Mechanical Effects of Vegetation	43
2.5.2	Hydrological Effects of Vegetation	59
2.5.3	Modelling the Effects of Vegetation on Slope Stability	63
2.6	Summary	66
3	DESCRIPTION OF THE CASE HISTORY	68
3.1	Background	68
3.2	Geomorphology and Geology of the area	68
3.2.1	Geomorphology of the area	68
3.2.2	Geology of the area	70
3.3	Description of the Landslide	72
3.3.1	Failure mechanism	72
3.3.2	Historic Events	72
3.4	Implementation of Mitigation measures	73
3.5	Site Investigation Programme at Badulusirigama	73
3.6	Monitoring programme Conducted at the site	77
3.7	Interpretation of the Investigation Results	78
3.8	Stability Assessment Conducted	79

3.9	Design of Mitigation Measures	80
3.10	Future studies	84
3.11	Summary	84
4	DEVELOPMENT OF THE GEOTECHNICAL MODEL OF THE BADULUSIRIGAMA SITE	86
4.1	Background	86
4.2	Assessment of investigation data.....	86
4.2.1	Contour Survey	86
4.2.2	Borehole Survey	88
4.2.3	Deduction of the subsurface profile	88
4.2.4	Determination of the active slip surface.....	89
4.2.5	Ground water table.....	93
4.3	Geotechnical model of the Badulusirigama site.....	93
4.4	Summary	94
5	TWO DIMENSIONAL PLANE STRAIN ANALYSIS OF THE BADULUSIRIGAMA LANDSLIDE	95
5.1	Background	95
5.2	Two dimensional plane strain idealization of the site profile	95
5.3	Seepage analysis using SEEP/W module	96
5.3.1	Soil water characteristic curve (SWCC).....	96
5.3.2	Hydraulic conductivity function (HCF)	97
5.4	Idealization of the cross section A1-A2 in SEEP/W.....	98

5.4.1	Boundary conditions	99
5.5	Initial ground water table condition of the slope (prevailing condition)	100
5.5.1	Initial pore water pressure profile derived from the ground water flow analysis.....	101
5.6	Stability of the slope with high ground water table condition	101
5.6.1	Definition of the slip surfaces.....	102
5.6.2	Results of the stability analysis for prevailing conditions	103
5.7	Modelling the effectiveness of rectification measures	105
5.7.1	Subsurface drainage improvement	105
5.7.2	Simulation of subsurface drainage improvement in two dimensional plain strain formation.....	105
5.7.3	Transient seepage analysis of the slope after drainage improvement	107
5.7.4	Performance of the rectified slope with subsurface drainage system during a typical rainfall event	116
5.8	Analysis of the composite slip surface	119
5.8.1	Improvement of stability along the composite slip surface with the installation of subsurface drainage measures	121
5.9	Effect of surface vegetation on the stability of the slope	126
5.9.1	Selection of the plant species.....	127
5.9.2	Root tensile strength tests	127
5.9.3	Modelling the root tensile strength.....	129
5.9.4	Hydraulic conductivity of the vegetated layer	130
5.9.5	Stability analysis of the vegetated slope.....	130

5.9.6	Response of the hybrid mitigation system to an actual rainfall event	132
5.10	Summary	135
6	DEVELOPING A THREE DIMENSIONAL FINITE ELEMENT MODEL OF THE BADULUSIRIGAMA LANDSLIDE	137
6.1	Background	137
6.2	PLAXIS 3D software for stability modelling.....	137
6.3	Development of three dimensional profile for the model	139
6.4	Model boundary conditions	141
6.4.1	Displacement boundary conditions	141
6.4.2	Flow boundary condition.....	142
6.5	Finite element mesh	143
6.6	Establishment of initial geo-static stresses of the ground	144
6.7	Analysis of the stability of the slope	147
6.7.1	Stability of the slope under prevailing conditions.....	147
6.8	Performance of the slope after implementation of fan –type subsurface drainage.....	150
6.8.1	Variation of the water table and stability of the slope under Case 2	155
6.8.2	Variation of the water table and stability of the slope under Case 3	157
6.8.3	Comparative improvement of the factor of safety under two cases.....	158
6.9	Performance of the radial drainage network in response to an actual rainfall event	162
6.10	Commentary on the developed 3D finite element model for Badulusirigama landslide	166

6.11	Summary	167
7	CONCLUSIONS AND RECOMMENDATIONS	168
7.1	Conclusions.....	168
7.1.1	Two dimensional analysis.....	169
7.1.2	Three dimensional analysis.....	170
7.2	Recommendation for future work.....	172
	References.....	173

List of Figures

Figure 2.1: Types of slip surfaces (Knappett and Craig, 2012)	11
Figure 2.2 Free body diagram of the failure mass.....	12
Figure 2.3: (a) Circular slip surface with overlying soil mass subdivided into slices (Duncan et al., 2012), (b) Forces acting on a slice	12
Figure 2.4 Definition of grid and radius (Geo Slope manual /2012)	15
Figure 2.5 Definition of block specified method (Geo Slope manual /2012).....	15
Figure 2.6: Typical nodal displacement curve (Donald and Giam 1988).....	18
Figure 2.7 (a): A homogeneous simple slope composed of two soil layers	19
Figure 2.8: (a) Displacement vectors within the slope (b) Incremental displacement vectors at critical the critical factor of safety value (Donald and Giam 1988).....	19
Figure 2.9 Variation of number of discontinuities crosses by the drain depending on rain inclination (Cook et al. 2007).....	23
Figure 2.10 Modelled phreatic surfaces for a slope with no drains with different anisotropic ratios of permeability (Pathmanathan 2009)	23
Figure 2.11 Theoretical effectiveness of horizontal drains in terms of flux with respect to drain position (Rahardjo et al. 2012).....	24
Figure 2.12 Conceptual design for subsurface drains (Rahardjo et al., 2003).....	25
Figure 2.13 Influence of drain length on stabilizing effect of horizontal drains (Santoso et al., 2009)	27
Figure 2.14 Two arrays of horizontal drains used to lower the ground water table at Sandlake roadside, Oregon (Conforth 2005).....	29
Figure 2.15 Plan view of the horizontal drains installed in the slope (Kleppe and Denby, 1984).....	29

Figure 2.16: Plan view of the landslide area	30
Figure 2.17: Cross section along the shear plane	31
Figure 2.18 Layout of subsurface drains and deep wells at Watawala landslide (Chandler and Broise, 2000).....	32
Figure 2.19 Typical sub surface drain profile (Chandler and Broise, 2000)	33
Figure 2.20 Rainfall and discharges from the sub surface drains (Chandler and Broise, 2000)	34
Figure 2.21: Photograph taken at Watawala in June 2018.....	35
Figure 2.22: Subsurface profile and the failure surface established through BH investigation and proposed alignment of the sub surface drain	36
Figure 2.23 Outlet of a subsurface drain installed at the site	37
Figure 2.24 The slope models with drains installed at a spacing of S and simulated blanket drain Gjetvaj et al., (2009).....	38
Figure 2.25 Slope model for SEEP /W analysis (Gjetvaj et al., 2009)	39
Figure 2.26 Equipotential lines obtained from 2D and 3D seepage analysis for different drain lengths and spacing (Gjetvaj et al. 2009)	40
Figure 2.27 Effects of vegetation on slopes (Morgan and Rikson, 1995)	42
Figure 2.28 Schematic slope model showing slope – vegetation – atmosphere interacting phenomena (Elia et al. 2017).....	43
Figure 2.29 : Schematic diagram of the stresses in the root during shear (Dias et al., 2017)	45
Figure 2.30: Increase of soil shear strength due to the effects of roots (Copping and Richards, 1995).....	47

Figure 2.31 Comparison of model predicted and experimental values (experimental values are represented by open symbols and predicted values by continuous line, Pallewatta et al., 2018).....	49
Figure 2.32 Perspex boxes for preparation of samples (Ali and Osman 2008)	51
Figure 2.33 specially fabricated large shear box (Ali and Osman 2008).....	51
Figure 2.34 Values of cohesion for Vetiver at various sample depths (Ali and Osman 2008)	52
Figure 2.35 Values of cohesion at various sample depths for different plants (Ali and Osman 2008).....	52
Figure 2.36 Typical finite element mesh assumed for analysis (Chok et al. 2015).....	54
Figure 2.37: Slope with the vegetation at different locations (Chok et al. 2015).....	55
Figure 2.38: Variation of FoS with the depth of root zone (Chok et al. 2015).....	56
Figure 2.39: Variation of FoS with root cohesion for different effective cohesion (Chok et al. 2015)	56
Figure 2.40 Variation of FoS of the vegetated slopes with different effective friction angle (Chok et al. 2015).....	57
Figure 2.41 Root anchoring in two different soil profiles (Cebada 2017).....	58
Figure 2.42 Rate of infiltration into a bare area and a vegetated area of slope (Zhan et al. 2007)	61
Figure 2.43 Pore water pressure variations with rainfall at different depths for (a) bare slope, (b) Slope with Orange and Jasmine, (c) Slope with Vetiver grass, (d) FoS variation and (e) Rainfall intensity (Rahardjo et al., 2014)	62
Figure 2.44 Variation of factor of safety variation for different slope for one day rainfall intensity of 22 mm/hour (Rahardjo et al., 2014)	63
Figure 2.45 Root configuration used in the analysis (Dharmasena and Kuathilaka , 2015)	65

Figure 3.1 Location of the landslide (Balasuriya and Nishikawa, 2016).....	69
Figure 3.2 Aerial Photograph of Badulusirigama area (JICA Report, September 2015)	70
Figure 3.3 Geological complexes of Sri Lanka and location of Badulusirigama site ...	70
Figure 3.4 Geology map of the area	71
Figure 3.5 Selected investigation locations and survey lines (JICA Report, September 2015)	75
Figure 3.6 Cross section along A1- A2 (JICA Report, September 2015).....	76
Figure 3.7 Cross sections along B1-B2 and C1-C2 (JICA Report, September 2015)...	76
Figure 3.8 Resistivity contours on a long section along the landslide (JICA Report, September 2015)	77
Figure 3.9 Plan View of the subsurface drainage improvement (JICA Report, September 2015).....	82
Figure 3.10 Sectional view of subsurface drains (JICA Report, September 2015)	83
Figure 4.1: Contour survey plan of the area	87
Figure 4.2: Subsurface profile through section A1- A2	89
Figure 4.3 Graph pertaining to pipe strain gauge at BB 01	91
Figure 4.4 Inclinator graph pertaining to BB 04	92
Figure 4.5 Inclinator graph pertaining to BB 02	92
Figure 4.6: Graphs pertaining to extensometers (SB 01, 02, 03 and 04)	93
Figure 5.1: SWCC used in the analysis (a) Colluvium; (b) Completely to highly weathered rock	98
Figure 5.2: HCF"s used in the analysis (a) Colluvium; (b) Completely to highly weathered rock	98

Figure 5.3 Cross section idealized in SEEP/W	99
Figure 5.4 Model showing the assigned boundary conditions	100
Figure 5.5 Variation of pore water pressure profile of the slope	101
Figure 5.6 The slip surfaces of each slide are fully defined in SLOPE/W module	103
Figure 5.7 Safety margins of the upper slide.....	104
Figure 5.8 Safety margins of the middle slide.....	104
Figure 5.9 Safety margins of the lower slide.....	104
Figure 5.10 Drains simulated in the section A1 - A2.....	106
Figure 5.11 Variation of flux rate through drains for Case 1	110
Figure 5.12 Comparison of the water flux into the drains for Case 2.....	110
Figure 5.13 Comparison of the water flux into the drains for Case 3.....	111
Figure 5.14 Variation of factor of safety of three slides under Case 2	112
Figure 5.15 Variation of factor of safety of three slides in Case 3	112
Figure 5.16 Percentage improvement in factor of safety of each slide with sequential construction under Case 2.....	113
Figure 5.17 Percentage Improvement in factor of safety of each slide under sequential construction in Case 3.....	114
Figure 5.18 Rainfall event from 1st December to 31st December 2014.....	117
Figure 5.19 Variation of factor of safety of the upper slide after subsurface drainage improvement due a critical rainfall event.....	118
Figure 5.20 Definition of the composite slip surface using block specified method...	120
Figure 5.21 Factor of safety of the composite slip surface under existing conditions	120

Figure 5.22 Variation of factor of safety of the composite slip surface under Case 2 & 3.....	122
Figure 5.23 Variation of the pore water pressure profile before and after subsurface drainage improvements: (a) Section IJ, (b) Section KL.....	123
Figure 5.24 Variation of factor of safety of the composite slip corresponding to rainfall event from 1st to 31st December 2014.....	125
Figure 5.25 Most critical failure surface during the rainfall event	125
Figure 5.26 Pore water pressure variation during the actual rainfall event: (a) Section IJ, (b) Section KL.....	126
Figure 5.27 Root tensile strength testing using Dynamometer	129
Figure 5.28 Slope profile after introducing the effect of vegetation.....	131
Figure 5.29 Critical slip surface after implementing hybrid measures	132
Figure 5.30 Variation of the factor of safety after implementing hybrid mitigation measures, corresponding to an actual rainfall event	133
Figure 5.31 Variation of the pore pressure profile of the slope after implementing hybrid measures under the actual rainfall event: (a) Section IJ, (b) Section KL.....	134
Figure 6.1 PLAXIS 3D model of Badulusirigama Landslide	140
Figure 6.2 Model from the different viewpoints (a) top view, (b) bottom view, (c) front view, (d) back view.....	141
Figure 6.3: Displacement boundary conditions assigned for the model	142
Figure 6.4 Variation of the ground water head within the shown in a cross section along the centre line of the slope	143
Figure 6.5 Variation of the pore water pressure along the section PQ	143
Figure 6.6 Finite element mesh generated for the problem	144
Figure 6.7 Initial principal stress profile of the site	146

Figure 6.8: Deviatoric shear strains along the failure plane.....	147
Figure 6.9: Vectors showing incremental displacements in the moving mass	148
Figure 6.10: Shadings indicating the incremental displacements in the moving mass (a) along the section PQ, (b) 3D view	148
Figure 6.11: Plot showing the initial factor of safety of the slope	149
Figure 6.12 Spatial arrangement of the subsurface drainage system (a) top view, (b) front elevation, (c) perspective view	153
Figure 6.13: Variation of the ground water table with drainage improvement: Case 2- top down (a) Initial, (b) DA, (c) DAB, (d) DABC, (e) DABCD, (f) DABCDE, (f) DABCDEF	156
Figure 6.14: Variation of the ground water table with drainage improvement: Case 2 - Bottom up (a) Initial, (b) DF, (c) DFE, (d) DFED, (e) DFEDC, (f) DFEDCB, (f) DFEDCBA	157
Figure 6.15: Variation of factor of safety pertaining to Case 2	159
Figure 6.16: Variation of factor of safety pertaining to Case 3	160
Figure 6.17 Plot showing the variation of factor of safety with time under two cases	161
Figure 6.18: Variation of the factor of safety of the slope corresponding to actual rainfall event	163
Figure 6.19: Variation of ground water table due to critical rainfall event: (a) Day 18, (b) Day 23, (c) 38, (d) Day 43, (e) Day 53	165

List of Tables

Table 2.1 Comparison of factor of safety equations (Fredlund and Krahn 1977)	14
Table 2.2 FoS variation with the location of root zone (Chock et al. 2015).....	54
Table 3:1 Instrumentation at the site	78
Table 3:2 Summary of the initial stability assessment (JICA Report, September 2015)	80
Table 3:3: Summary of design countermeasure (JICA Report, September 2015)	84
Table 4.1 Details about the location, depth of termination and depth to the ground water table at borehole locations	88
Table 4.2: Interpreted subsurface soil profile of Badulusirigama landslide	89
Table 4.3: Summary of information from monitoring instrumentation	90
Table 4.4: Summary of soil parameters used for different layers	94
Table 5:1: Soil properties for seepage analysis	97
Table 5:2: details of the subsoil /rock layers drawn in SEEP/W	99
Table 5:3: Description of the boundary conditions assigned	100
Table 5:4: Material properties assigned for stability analysis.....	102
Table 5:5: Factor of safety values for prevailing conditions.....	103
Table 5:6: Summary of the subsurface drains constructed at the site.....	105
Table 5:7: Details of the drains simulated in Figure 5.10	107
Table 5:8: Summary of drain installation sequence under each case considered	108
Table 5:9: Percentage Improvement of factor of safety after drainage improvement under Case 2.....	113

Table 5:10: Variation of factor of safety after drainage improvement under Case 3 ..	114
Table 5:11: Variation of factor of safety of the composite slip under Case 2 and Case 3.....	121
Table 5:12: Root tensile strength of selected species.....	128
Table 5:13: Root cohesion of Clove tree for different spacing's	130
Table 5:14: Modified soil strength parameters after introducing vegetation.....	131
Table 5:15 Variation of factor of safety after improving subsurface drains and vegetation.....	132
Table 6:1: Notation used for different subsurface layers in 3D model.....	140
Table 6:2: Details of the finite element mesh.....	144
Table 6:3: Spatial arrangement of the drainage system in 3D formulation	151
Table 6:4: Sequence of simulation under the each case considered.	154
Table 6:5: Percentage variation of the factor of safety for the two construction cases	160
Table 6:6 Percentage variation of factor of safety	163