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COMPARISON OF PORTAL FRAMES OF UB SECTIONS WITH THE LATTICE GIRDER PORTAL FRAMES.

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SENANI JAYASINGHE
B.Sc. Eng (Hons)



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*This thesis is submitted to the Department of Civil Engineering
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ABSTRACT

The first part of the thesis is devoted to the study of the properties of the solutions of the Dirichlet problem for the Laplace equation in the domain bounded by the unit circle. The second part is devoted to the study of the properties of the solutions of the Dirichlet problem for the Laplace equation in the domain bounded by the unit circle. The third part is devoted to the study of the properties of the solutions of the Dirichlet problem for the Laplace equation in the domain bounded by the unit circle.

DECLARATION

This work included in this thesis in part or in whole has not been submitted for any other academic qualification at any other institution

ABSTRACT

Portal frame structures are very popular in industrial buildings due to their pleasing appearance and shorter construction period. In Sri Lanka too, steel construction is increasingly being used for single storey buildings. Basically there are two types of steel portal frames adopted in Sri Lanka; these are called Prefabricated and pre-Engineered portal frames.

Although the portal frame is inherently pleasing to the eye, given a well proportioned and detailed design, and less in cost for providing controlled environments, industrial connotation, together with increased service requirements has encouraged the use of lattice trusses for the roof structures. The structural forms both in the simple form fixed column bases and as portal frames with moment resisting connections between the tops of columns are used for long span structures.

Although it is widely used, portal frame may not be the best solution for large span single story buildings. Lattice girders are successfully used for medium size spans. However it is not widely used for large span buildings in Sri Lanka.

The construction industry here has many years of experience and necessary skills on fabricating truss or lattice type structures. Even though skilled and unskilled labour involvement is high, it will not reflect crucially on ultimate cost of the structure as the labour cost is less here in Sri Lanka, compared to European countries. Therefore in Sri Lanka lattice girder construction for large span buildings may be more cost effective.

The main objective of this research is to compare the portal frames formed with lattice girders with the conventional portal frames with Universal Beam sections for different span ranges and compare them in same spans, in the context of optimum cost and space requirements.

The space requirement for the lattice girder column is not critical compared to the space requirement for the column in UB portal for all spans.

The governing criteria for selecting the section sizes of the frame members of the both structural forms depends on the wind condition prevailing in the location selected for building the structure.

The study is concluded as, for wind zone -1; the portals for selected geometry with angle sections are less economical when it is compared over the portal with universal beam sections for similar spans when the elastic analysis is performed for both structural forms.

For wind zone-3, weight of the 30m and 40m portals with angle sections are less when it is compared with the portals with universal beam sections. The percentage reduction in weight ranges from 10-20 for 30m and 40m frames. For spans 20m and 50m, weight of the portals with angle sections are higher when it is compared with the portals with universal beam sections.

A uniform pattern of weight variation cannot be observed in Wind Zone-3 structures as in Wind-Zone-1.

Hence for Wind Zone-3, for selecting economical structural forms, a detailed cost analysis should be carried out for both structural forms inclusive of the fabrication cost.

Further, since this study has been limited to lattice girder portals of a specific geometry, it is recommended that if the geometry of a particular structure to be designed is very different, the cost effectiveness needs to be determined.

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ABBREVIATIONS

A	=	Area or Surface area of the structural element of which the wind surf
A _e	=	Effective area
A _g	=	Gross area
A _o	=	Area of the rectilinear element of the section which has the largest dimension in the direction parallel to the load
A _p	=	Apex
A _s	=	Shear area of a bolt
A _v	=	Shear area
b	=	Outstand or width of panel
C _{pe}	=	External wind pressure coefficient
C _{pi}	=	Internal wind pressure coefficient
D	=	Depth of section
d	=	Depth of web or Nominal diameter of a bolt
E	=	Eaves point
F	=	Wind force or Applied force (compressive or tensile)
FS	=	Frame spacing
F _v	=	Shear force
h	=	Height to eaves of the building
I	=	Intermediate point
K	=	Constant found in design of building of high winds in Sri Lanka
L	=	Length of Building
L _{xx} , L _{yy}	=	Effective length in x-x and y-y direction.
l	=	Greater horizontal dimension of the building
M	=	Mass of roof covering or Applied moment at the section considered
m	=	Equivalent uniform moment factor
M _b	=	Buckling resistance moment (lateral torsional I capacity) about major axis
M _c	=	Moment capacity
M _x	=	Applied moment about the major axis
M _y	=	Applied moment about the minor axis
M _{cx} , M _{cy}	=	Reduced moment capacity of the section about the major and minor axis in the presence of axial load
M ₂	=	Moment about local axis (2) with respect to the local axes of frame element in SAP 2000.8

M3	=	Moment about local axis (3) with respect to the local axes of frame element in SAP 2000.8
NE ₁	=	Next to eaves point - 1
NE ₂	=	Next to eaves point - 2
P	=	Axial force (Tension or compression) in local axis (1) with respect to local axes of frame element in SAP 2000.8
P _b	=	bearing capacity of a bolt
P _{bb}	=	Bearing strength of a bolt
P _c	=	Compressive capacity
p _c	=	Compressive strength
P _{cs}	=	Compressive strength for slender section
p _s	=	Shear strength of a bolt
P _s	=	Shear capacity of a bolt
P _v	=	Shear capacity
P _y	=	Design strength of steel
q	=	Dynamic wind pressure
R1	=	Rotation of a joint about the joint local axis (1) with respect to the joint local axes of frame element in SAP 2000.8
R2	=	Rotation of a joint about the joint local axis (2) with respect to the joint local axes of frame element in SAP 2000.8
R3	=	Rotation of a joint about the joint local axis (3) with respect to the joint local axes of frame element in SAP 2000.8
r _x , r _y , r _{xx} , r _{yy}	=	Radius of gyration about xx and yy axes.
S	=	Span of the frames in plan
S _x , S _y	=	Plastic modulus about major and minor axis
S/2	=	Half space length of the roof in plan
S1	=	Topography factor
S2	=	Ground roughness
S3	=	Statistical factor
T	=	Torque about local axis (1) with respect to the local axes of frame element in SAP 2000.8
T	=	Thickness of flange
t	=	Thickness of web or thickness of the connected ply of a connection
U1	=	Displacement of a joint in the direction of joint local axis (1) with respect to the joint local axes of frame element in SAP 2000.8
U2	=	Displacement of a joint in the direction of joint local axis (2) with respect to the joint local axes of frame element in SAP 2000.8

U_3	=	Displacement of a joint in the direction of joint local axis (3) with respect to the joint local axes of frame element in SAP 2000.8
V	=	Basic wind speed
V_s	=	Design wind speed
V_2	=	Shear force in local axis (2) with respect to local axes of frame elements in SAP 2000.8
V_3	=	Shear force in local axis (3) with respect to local axes of frame element in SAP 2000.8
w	=	Lesser horizontal dimension of the building
Z	=	Smallest modules of elasticity about the appropriate axis or lever Arm
Z_x, Z_y	=	Elastic modulus about major and minor axis
α°	=	Wind direction
θ°	=	Roof angle in degrees
λ_{TB}	=	Equivalent slenderness
λ	=	Slenderness, that is effective length divided by the radius of gyration

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